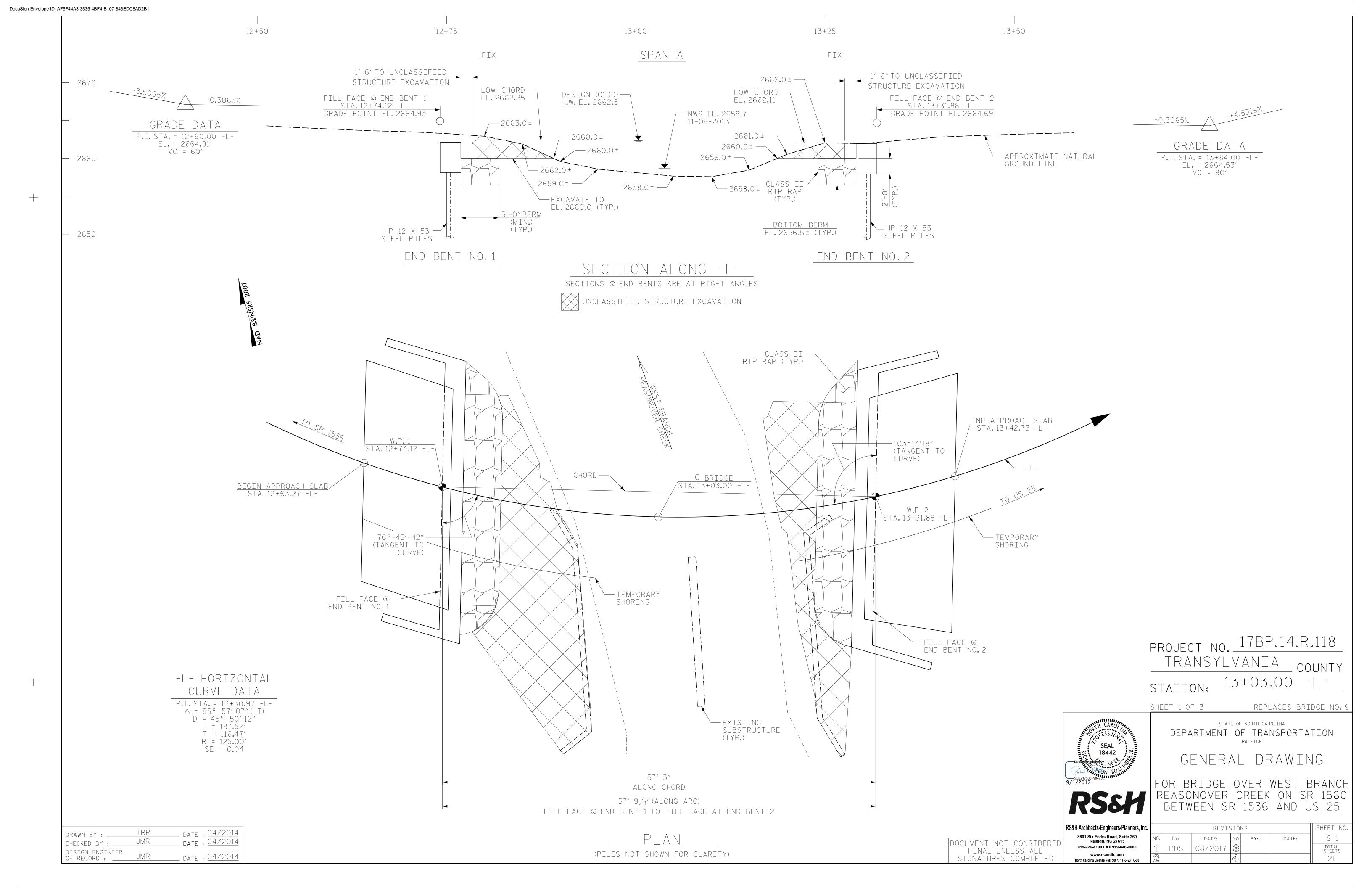
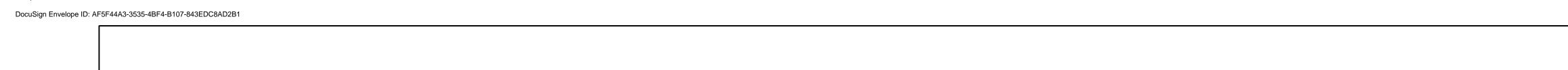
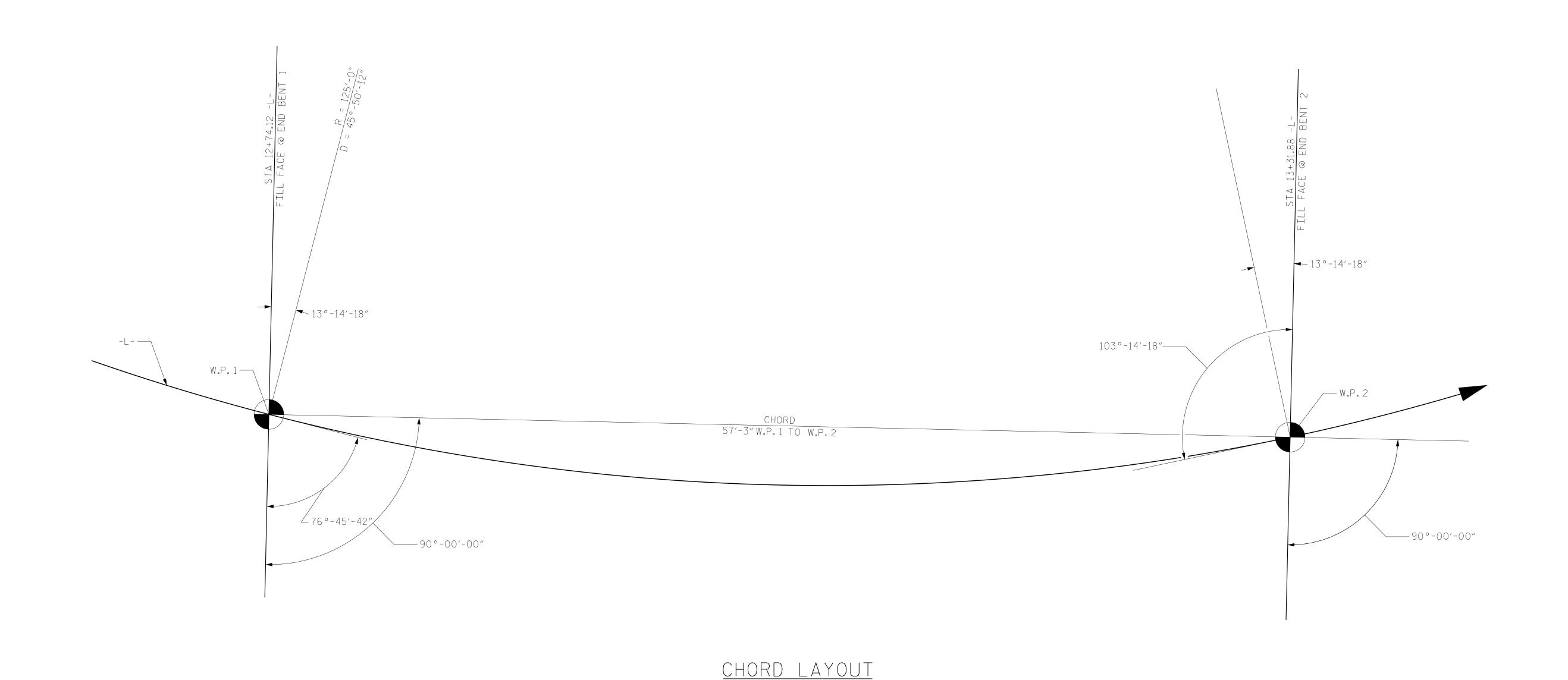
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NOTE: END BENTS ARE PARALLEL

PROJECT NO. 17BP.14.R.118

TRANSYLVANIA COUNTY

STATION: 13+03.00 -L-

SHEET 2 OF 3



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER WEST BRANCH REASONOVER CREEK ON SR 1560 BETWEEN SR 1536 AND US 25

BY: DATE:

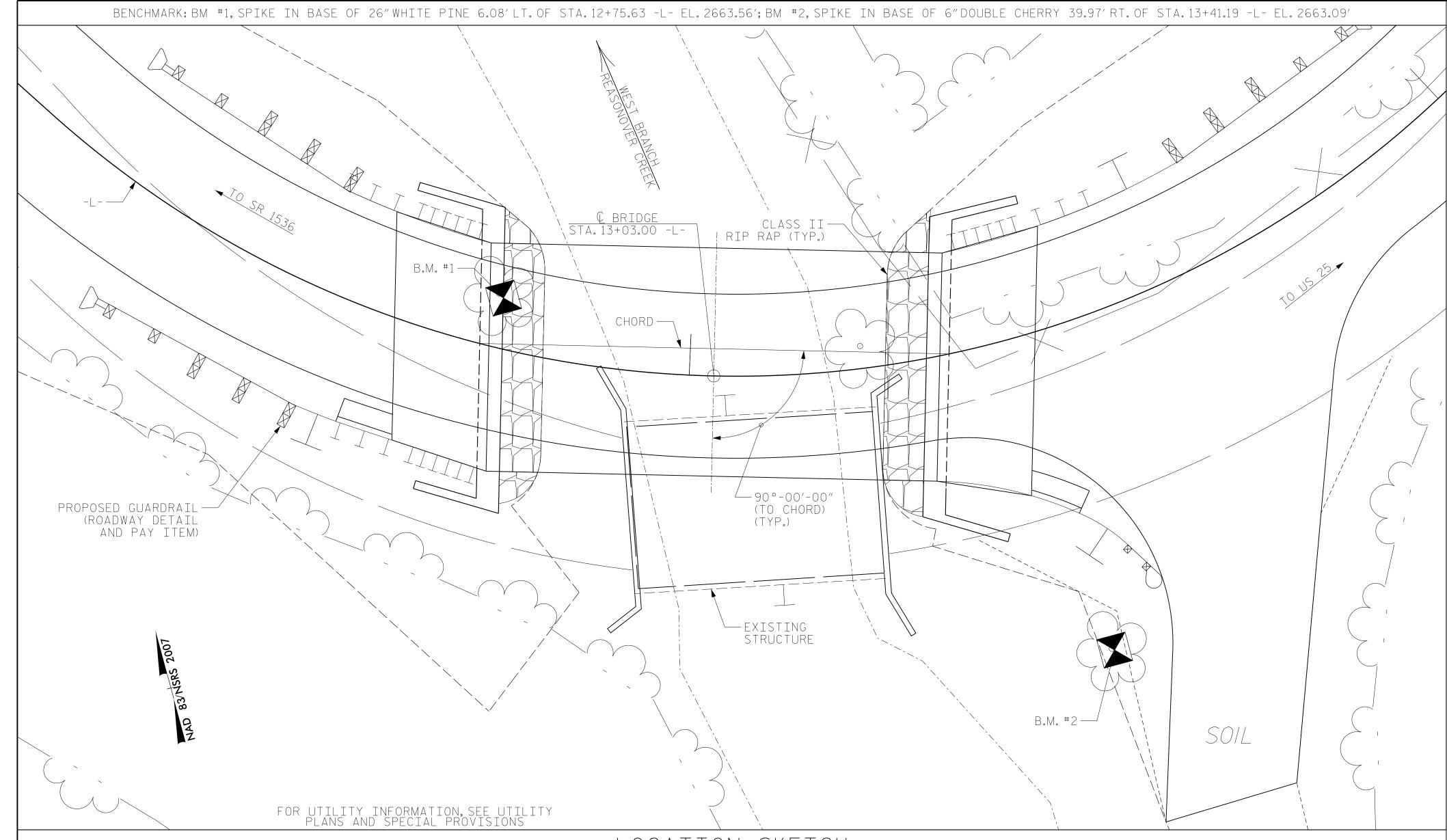
SHEET NO. S-2

TOTAL SHEETS

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itects-Engineers-Planners, Inc.			REVIS	SIC
Six Forks Road, Suite 260 Raleigh, NC 27615	NO.	BY:	DATE:	NO
26-4100 FAX 919-846-9080	1	PDS	08/2017	3
www.rsandh.com rolina License Nos. 50073 * F-0493 * C-28	2			4

DRAWN BY :	TRP JMR	DATE: 03/2014 DATE: 04/2014
DESIGN ENGINEER	JMR	DATE . 04/2014



LOCATION SKETCH

#### TOTAL BILL OF MATERIALS 3'-0" X 1'-9" VERTICAL PILE DRIVING UNCLASSIFIED BRIDGE STEEL RIP RAP GEOTEXTILE REMOVAL OF CLASS A REINFORCING HP 12 X 53 CONCRETE ELASTOMERIC PRESTRESSED ASBESTOS EQUIPMENT STRUCTURE APPROACH PILE EXCAVATION EXCAVATION CLASS II EXISTING FOR STEEL PILES CONCRETE STEEL BARRIER BEARINGS CONCRETE CORED | ASSESSMEN NOT IN SOIL EXCAVATION SLABS SETUP IN SOIL (3'-6" THICK) STRUCTURE DRAINAGE RAIL SLABS CU. YDS. | LUMP SUM LBS. NO. LIN.FT. NO. LIN. FT. LUMP SUM LUMP SUM EACH LIN.FT. LIN.FT. LIN.FT. LUMP SUM LUMP SUM NO. TONS SQ. YDS. SUPERSTRUCTURE LUMP SUM 110.0 LUMP SUM 550 LUMP SUM END BENT 1 LUMP SUM 20.7 2505 75 38 15 15 42.2 LUMP SUM END BENT 2 20.7 2508 75 35 39 43.3 15 LUMP SUM LUMP SUM LUMP SUM 5013 10 LUMP SUM TOTAL LUMP SUM 41.4 10 150 30 50 110.0 77 85.5 550

FOUNDATION NOTES: FOR PILES, SEE GEOTECHNICAL SPECIAL PROVISIONS AND SECTION 450 OF THE

PILES AT END BENTS NO.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENTS NO.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.1, LEFT. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

DRILLED-IN PILES ARE REQUIRED FOR END BENT NO. 1, RIGHT. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 2648 FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS

DRILLED-IN PILES ARE REQUIRED FOR END BENT NO. 2, LEFT AND RIGHT. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 2648 FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

CONCRETE OR GROUT IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENTS NO.1 AND 2.

> DOCUMENT NOT CONSIDEREI FINAL UNLESS ALL SIGNATURES COMPLETE

# NOTES:

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET 1 OF 3 SHALL BE EXCAVATED FOR A DISTANCE OF 12 FT LEFT AND 36 FT RIGHT OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS, FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC. SEE ROADWAY PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF ONE 15'-1" SPAN AND ONE 15'-6" SPAN, ON A TIMBER DECK ON TEN STEEL I-BEAMS, ON A TIMBER CAP WITH TIMBER PILE AND TIMBER END BENTS WITH A CLEAR ROADWAY WIDTH OF 19'-7"LOCATED UPSTREAM FROM THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+03.00 -L-.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."

# HYDRAULIC DATA

DESIGN DISCHARGE = 600 CFS FREQUENCY OF DESIGN FLOOD = 25 YRS DESIGN HIGH WATER ELEVATION = 2662.3 DRAINAGE AREA = 1.73 SQ. MI. BASE DISCHARGE (Q100) = 850 CFS BASE HIGH WATER ELEVATION = 2662.46

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 1560 CFS FREQUENCY OF OVERTOPPING FLOOD = 100 YRS+ OVERTOPPING FLOOD ELEVATION = 2664.7

> 7BP.14.R.118 TRANSYLVANIA 13+03.00 -L-

SHEET 3 OF 3



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North Carolina License Nos. 50073 \* F-0493 \* C-28

DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

STATE OF NORTH CAROLINA

FOR BRIDGE OVER WEST BRANCH REASONOVER CREEK ON SR 1560 BETWEEN SR 1536 AND US 25

SHEET NO REVISIONS S-3 BY: DATE: DATE: NO. BY: TOTAL SHEETS PDS | 08/2017

STANDARD SPECIFICATIONS

DATE: <u>03/2014</u>

DATE : 04/2014

\_ DATE : <u>04/201</u>

TRP

JMR

JMR

DRAWN BY :

CHECKED BY : .

DESIGN ENGINEER

OF RECORD: \_\_

# LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE

				STRENGTH I LIMIT STATE						SE	RVICE	III	LIMI	T STA	TE									
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING Load Rating	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM Left end of Span (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM Left end of Span (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27	
DESIGN		HL-93(0pr)	N/A		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
		HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A						
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL .	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL .	27	
	_	SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL .	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55′	EL .	27	
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55′	EL	27	0.523	2.83	55′	EL	5.4	0.80	0.275	2.08	55′	EL	27	
	>	SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55′	EL	27	
		SNAGGRS4	34.925		1.189				1.73		EL ———	27	0.523			EL	5.4	0.80		1.19	55′	EL	27	
		SNS5A SNS6A	35.550 39.950		1.16	41.255	1.4	0.275	1.69 1.57	55′ 55′	EL EL	27	0.523	1.82 1.68	55′ 55′	EL EL	5.4 5.4	0.80	0.275	1.16	55′  55′	EL EL	27	
		SNS7B	42.000		1.028	43.102	1.4	0.275	1.5	55′	EL	27	0.523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
LEGAL LOAD		TNAGRIT3	33.000		1.32	43.556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
RATING		TNT4A	33.075		1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27	
		TNT6A	41.600		1.101	45.811	1.4	0.275	1.6	55′	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
		TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27	
		TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
		TNAGT5B	45.000	3	1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{ extsf{DW}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2.

3.

4

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\sqrt{3}$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.14.R.118

TRANSYLVANIA COUNTY

STATION: 13+03.00 -L-



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DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-4
		(F)			TOTAL SHEETS
		4			21

1
2
3

LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: TRP DATE: 04/2014 CHECKED BY: JMR DATE: 05/2014

DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

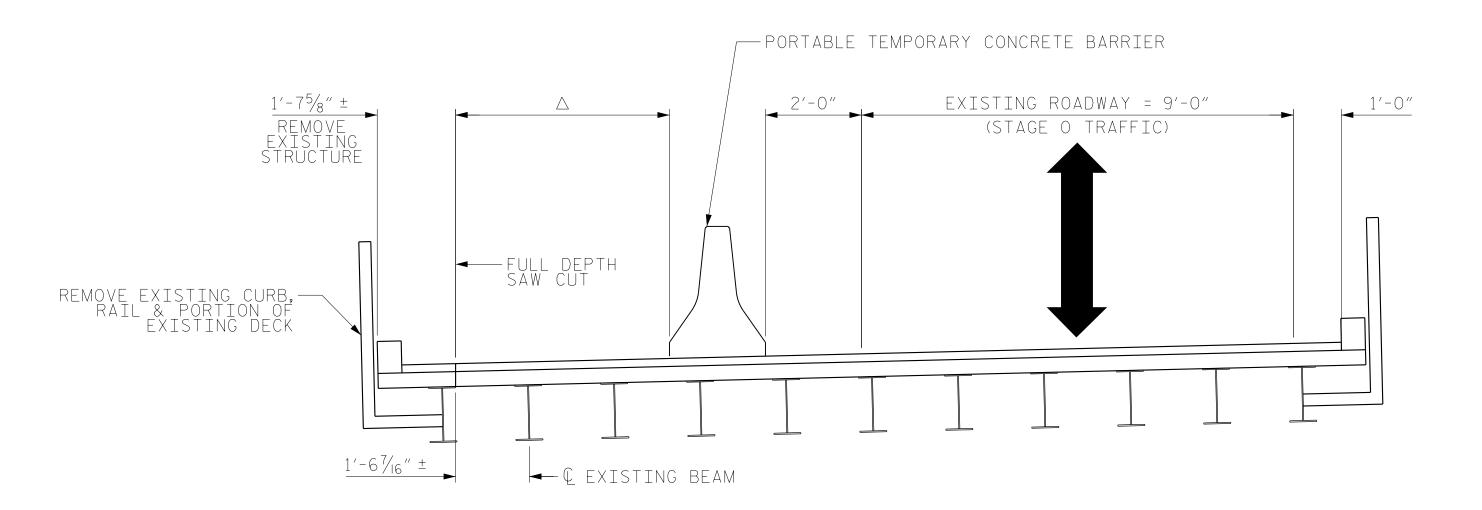
# NOTES:

FOR TRAFFIC PHASING, SEE TRAFFIC CONTROL PLANS.

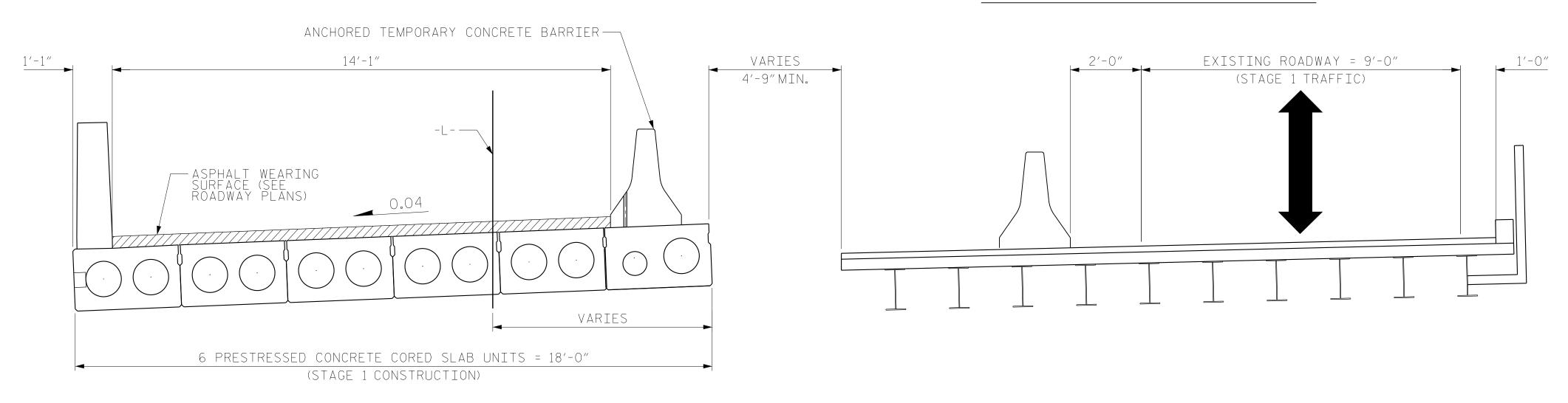
FOR STAGED CONSTRUCTION DETAILS, SEE ROADWAY PLANS.

THE TEMPORARY CONCRETE BARRIER USED IN STAGED CONSTRUCTION IS A ROADWAY DETAIL AND PAY ITEM.

A SEE TRAFFIC CONTROL PLANS FOR LOCATION OF THE PORTABLE CONCRETE BARRIER.

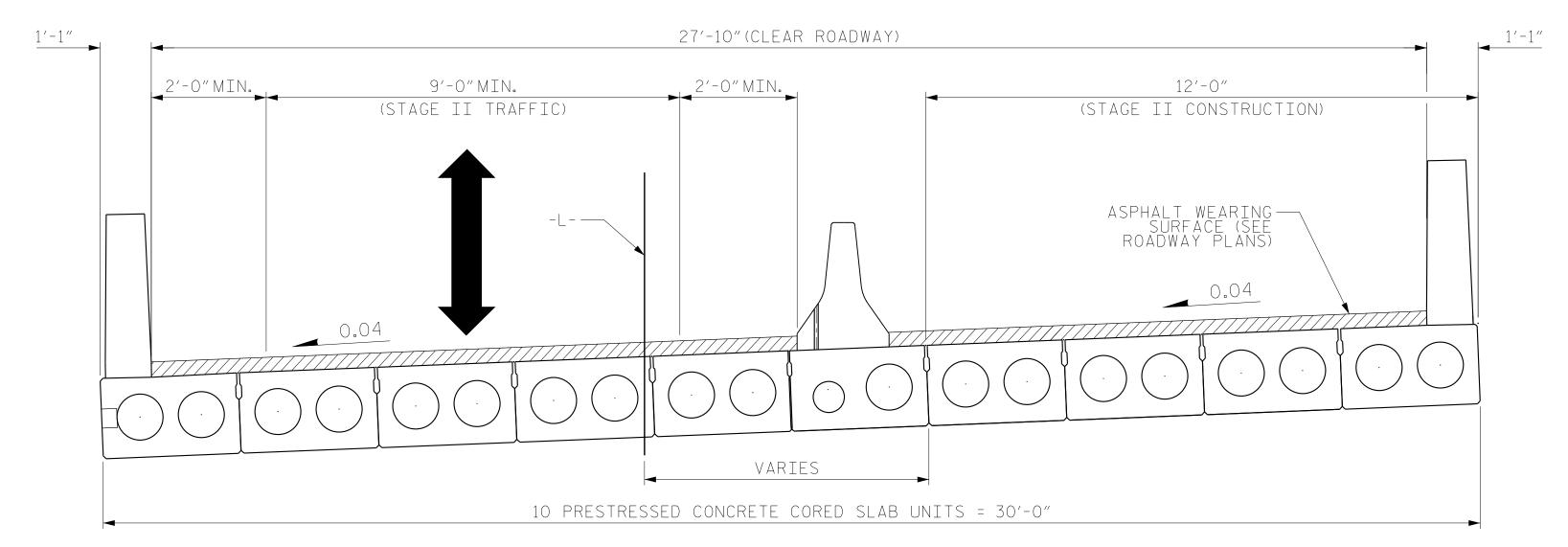


# STAGE O CONSTRUCTION



STAGE I CONSTRUCTION

STAGE I TRAFFIC



DRAWN BY: TRP DATE: 04/2014
CHECKED BY: JMR DATE: 05/2014
DESIGN ENGINEER OF RECORD: DATE: 05/2014

STAGE 1 TRAFFIC

STAGE II CONSTRUCTION

PROJECT NO. 17BP.14.R.118

TRANSYLVANIA COUNTY

STATION: 13+03.00 -L-



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DEPARTMENT OF TRANSPORTATION

RALEIGH

SUPERSTRUCTURE

CONSTRUCTION SEQUENCE

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, Inc.		SHEET NO.					
	NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
	1			3			TOTAL SHEETS
	2			4			21

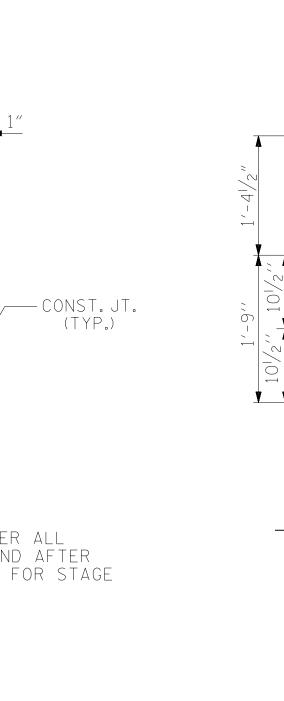
1'-9" TYP.)

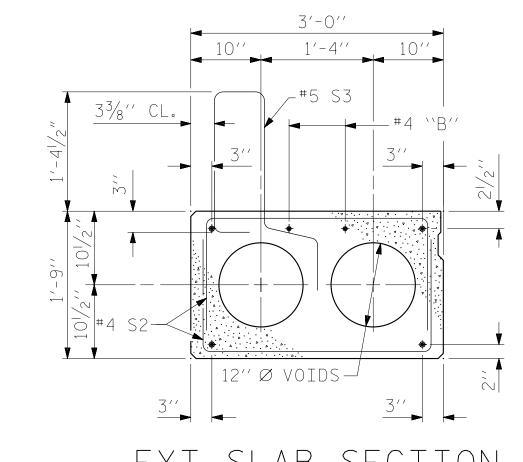
SEE "BRIDGE —

APPROACH SLAB'' SHEET FOR DETAILS

8"WIDE DECK DRAIN—

BLOCKOUT (TYP.)



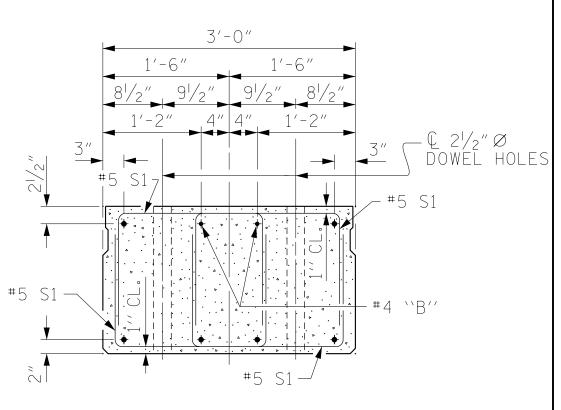


# EXT. SLAB

TYPE I & TYPE V (FOR PRESTRESSED STRAND LAYOUT, SEE "INTERIOR SLAB SECTION - TYPE II & IV") C TEMPORARY GUARDRAIL ANCHOR ASSEMBLY — (SEE NOTE) -

TYPE III (FOR PRESTRESSED STRAND LAYOUT, SEE "INTERIOR SLAB SECTION - TYPE II & IV")

NOTE: FOR LOCATION OF CONCRETE INSERTS, SEE "3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT DETAILS" SHEET 5 OF 6.



# END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

> PROJECT NO. <u>178</u>P.14.R.118 TRANSYLVANIA STATION: 13+03.00 -L-

> > STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

 $3'-0'' \times 1'-9''$ 

PRESTRESSED CONCRETE

CORED SLAB UNIT

90° SKEW

NO. BY:

SHEET 1 OF 6

SEAL 18442 9/1/2017

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# #4 \\B''-@ 2"CTS. @ 2"CTS. @ 2"CTS.

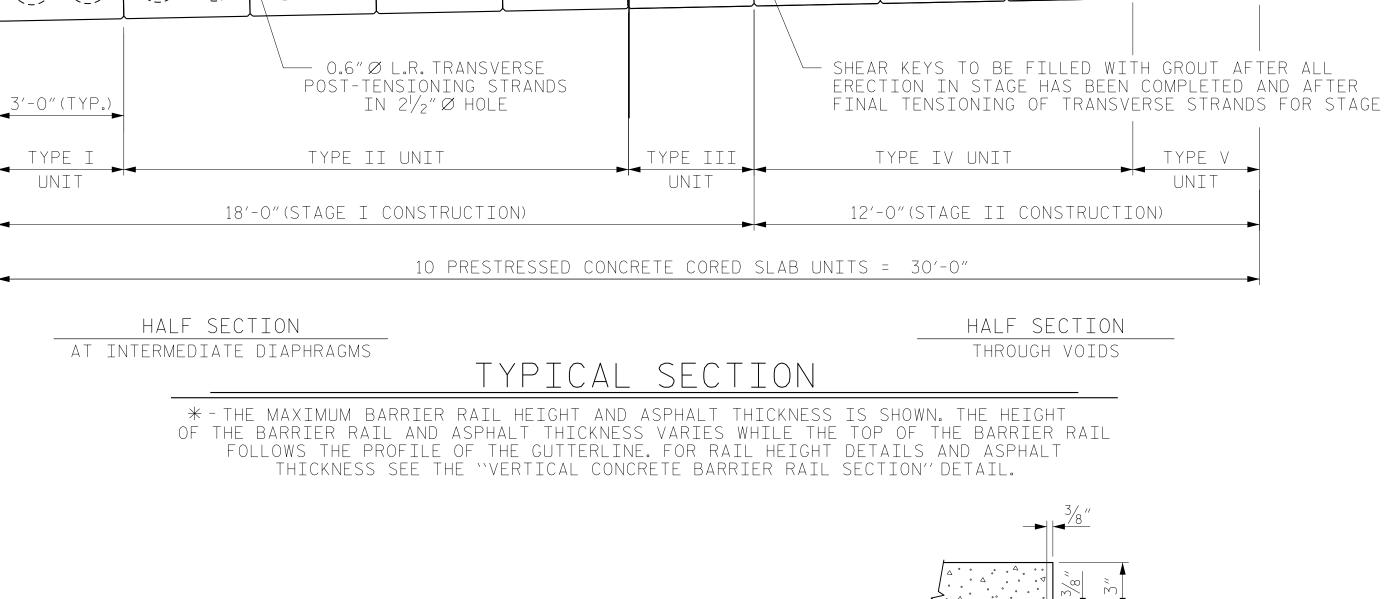
# INTERIOR SLAB SECTION (55' UNIT)

TYPE II & TYPE IV (19 STRANDS REQUIRED)

# 0.6'' Ø LOW RELAXATION STRAND LAYOUT

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



30'-0"

27'-10"(CLEAR ROADWAY)

VARIES  $12'-2\frac{3}{4}''$  TO  $15'-3\frac{1}{2}''$ 

2<sup>3</sup>⁄<sub>4</sub>" @ € BRG. —

0.04

(MIN.)

\_\_\_2¾″ @ Û BRG.

— ASPHALT WEARING

SURFACE (SEE ROADWAY PLANS)

(MIN.)

VARIES  $12'-6\frac{1}{2}$ " TO  $15'-7\frac{1}{4}$ "

-2<sup>3</sup>/<sub>4</sub>"@ & BRG.(MIN.)

FIXED END

 $-2^{1/2}$  Ø DOWEL HOLE

VOIDS-

ASPHALT WEARING

SURFACE —

VERTICAL CONCRETE BARRIER RAIL (TYP.)
FOR DETAILS SEE "VERTICAL
CONCRETE BARRIER RAIL SECTION"

GRADE PT.

2 LAYERS OF 30 LB. ROOFING FELT TO PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF PREVENT BOND. - ELASTOMERIC EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY BEARING PAD — SEE ''END BENT'' CONTRACTOR.— SHEETS FOR DETAILS SECTION AT END BENT

THREADED INSERT DETAIL

SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE

OF EXTERIOR CORED SLABS.

TRP DATE: 04/2014 ASSEMBLED BY : JMR CHECKED BY : DATE: 05/2014 DRAWN BY: DGE 5/09 REV. 12/11 MAA/AAC MAA/TMG CHECKED BY : BCH 6/09

11/2″Ø BACKER ROD-

TOTAL SHEETS

REVISIONS

DATE:

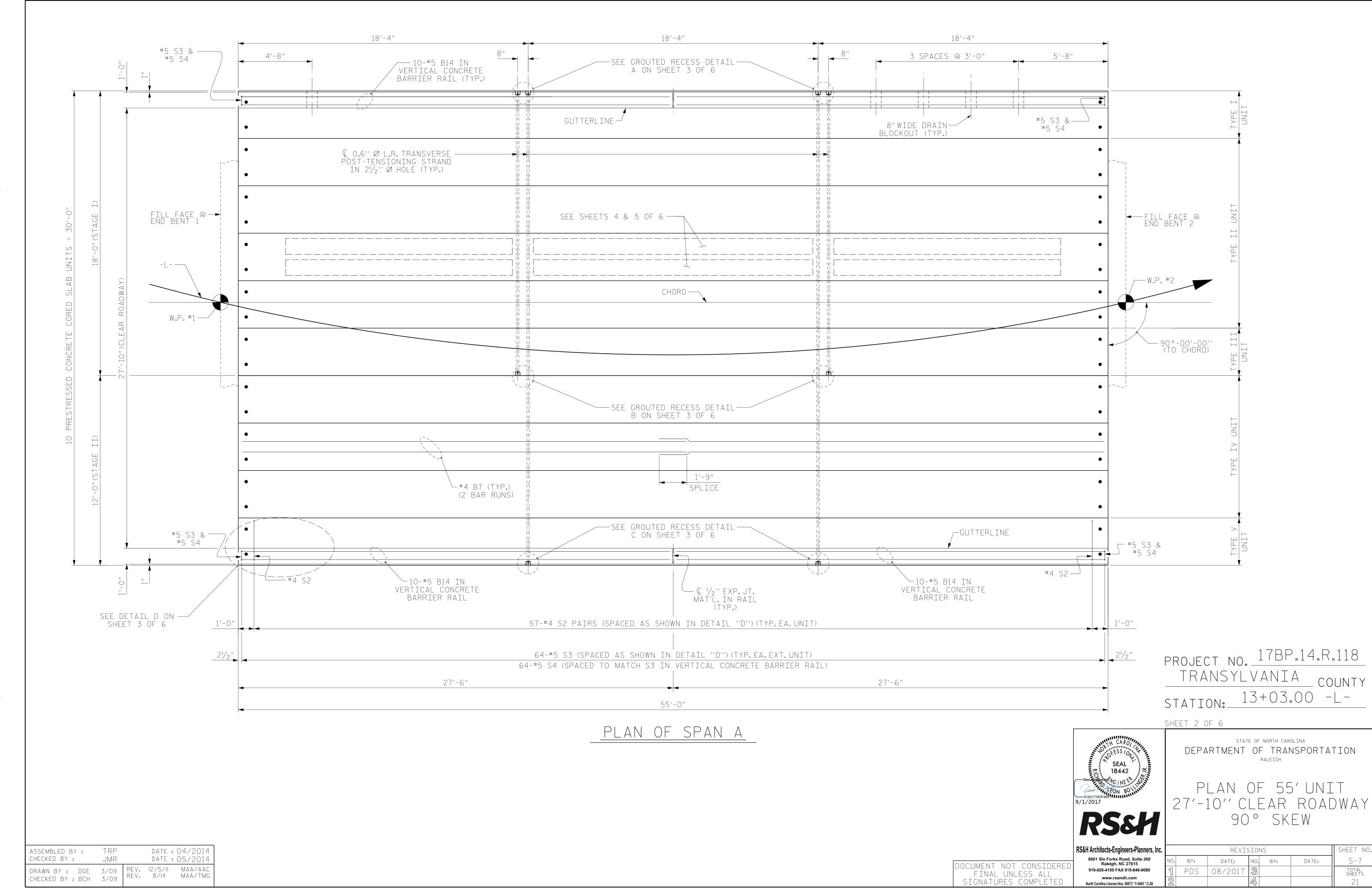
FINAL UNLESS ALL Signatures completed

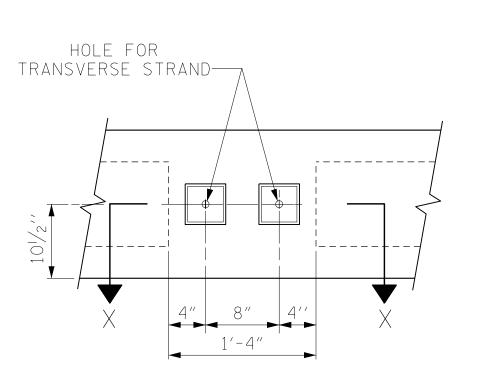
BY:

DATE:

SHEET NO

S-6





-● STRAND #2 € 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A NON-CORROSIVE ▲ STRAND #1. <u>\_\_\_\_5/8</u>′′ X 5′′ X 5′′ ₽ (TYP.) - STRAND VISE -FILL 51/4" RECESS WITH GROUT 1'-1|/4"

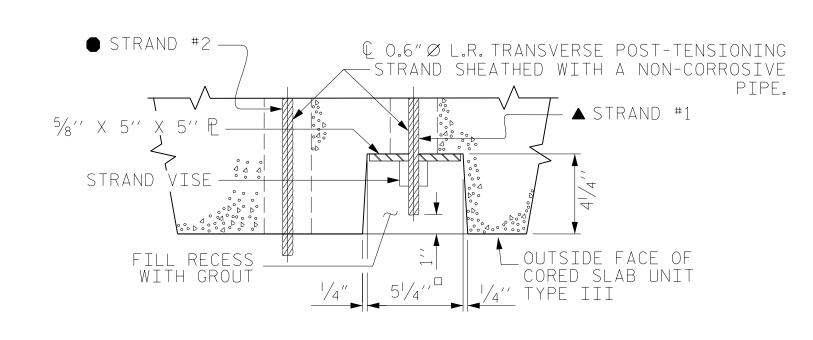
SECTION X-X

-----1'-4"

ELEVATION VIEW

HOLE FOR

---TRANSVERSE STRAND



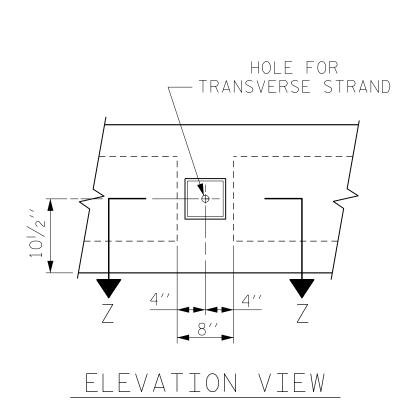
SECTION Y-Y

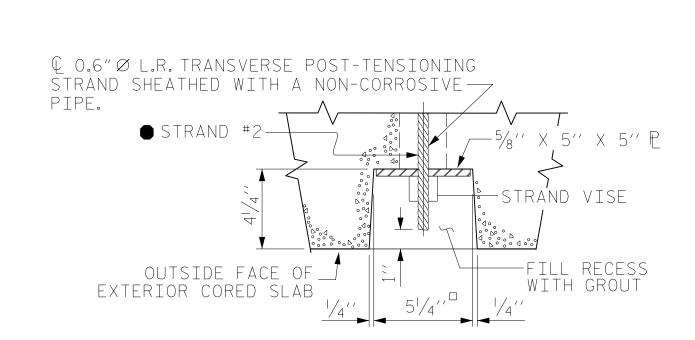
ELEVATION VIEW

DETAIL\_A (TYPE I UNIT)

DETAIL B

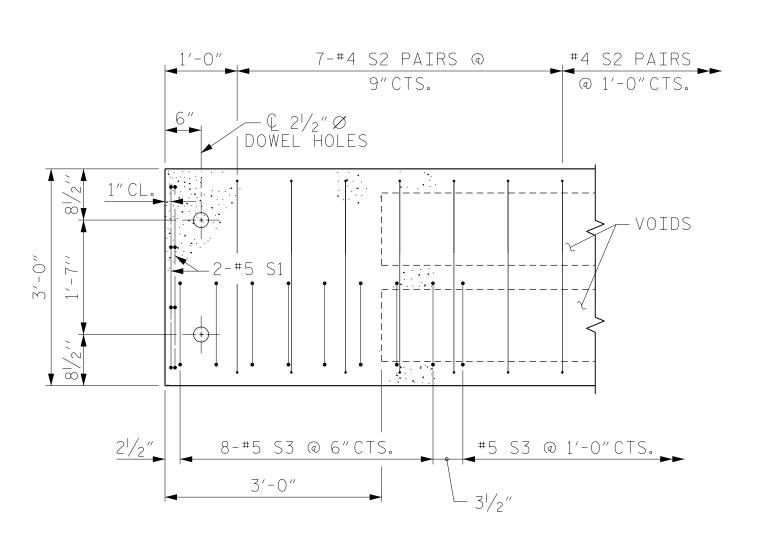
(TYPE III UNIT) (UPSTATION STRANDS SHOWN, DOWNSTATION STRANDS SIMILÁR)





SECTION Z-Z

DETAIL C (TYPE V UNIT)



DETAIL D (TYPICAL EACH END OF UNIT) Note: exterior unit shown - interior UNIT SIMILAR EXCEPT OMIT #5 S3 BARS.

DOCUMENT NOT CONSIDERED

FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT NO. <u>178P.14</u>.R.118 TRANSYLVANIA COUNTY STATION: 13+03.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

SHEET 3 OF 6



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STANDARD 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

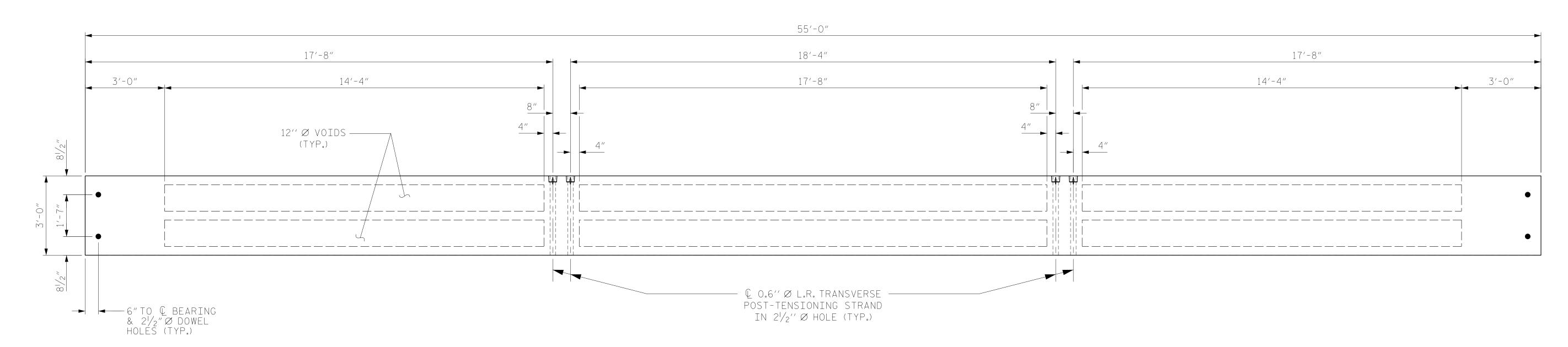
SHEET NO REVISIONS S-8 DATE: BY: DATE: NO. BY: TOTAL SHEETS

# GROUTED RECESS DETAILS

- ▲ STRAND #1 GOES THRU 6 CORED SLAB UNITS (TO BE TENSIONED DURING STAGE I CONSTRUCTION)
- STRAND #2 GOES THRU ALL 10 CORED SLAB UNITS (TO BE TENSIONED DURING STAGE II CONSTRUCTION)

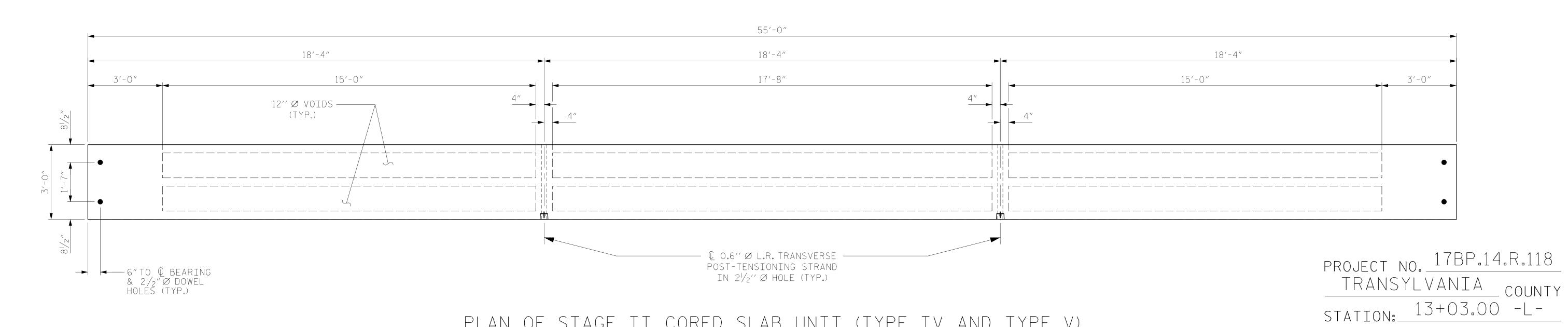
DATE: 04/2014 TRP ASSEMBLED BY : JMR DATE: 05/2014 CHECKED BY : DRAWN BY: DGE 5/09 REV. 12/11 MAA/AAC CHECKED BY: BCH 6/09 REV. 8/14 MAA/TMG

STD. NO. 21" PCS2\_30\_90S



# PLAN OF STAGE I CORED SLAB UNIT (TYPE I AND TYPE II)

(EXTERIOR UNIT (TYPE I) SHOWN, INTERIOR UNIT (TYPE II) SIMILAR EXPECT OMIT GROUTED RECESSES



PLAN OF STAGE II CORED SLAB UNIT (TYPE IV AND TYPE V)

(EXTERIOR UNIT (TYPE V) SHOWN, INTERIOR UNIT (TYPE IV) SIMILAR EXPECT OMIT GROUTED RECESSES

NOTES:

CORED SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.
SEE DETAIL "D", SHEET 3 OF 6 & PLAN VIEW SHEET 2 OF 6
FOR REINFORCEMENT DETAILS.

\_\_ DATE : <u>04/2014</u> \_\_ DATE : <u>05/2014</u> DRAWN BY : \_ CHECKED BY : \_\_ DESIGN ENGINEER OF RECORD : \_\_\_\_ \_ DATE : <u>05/2014</u>

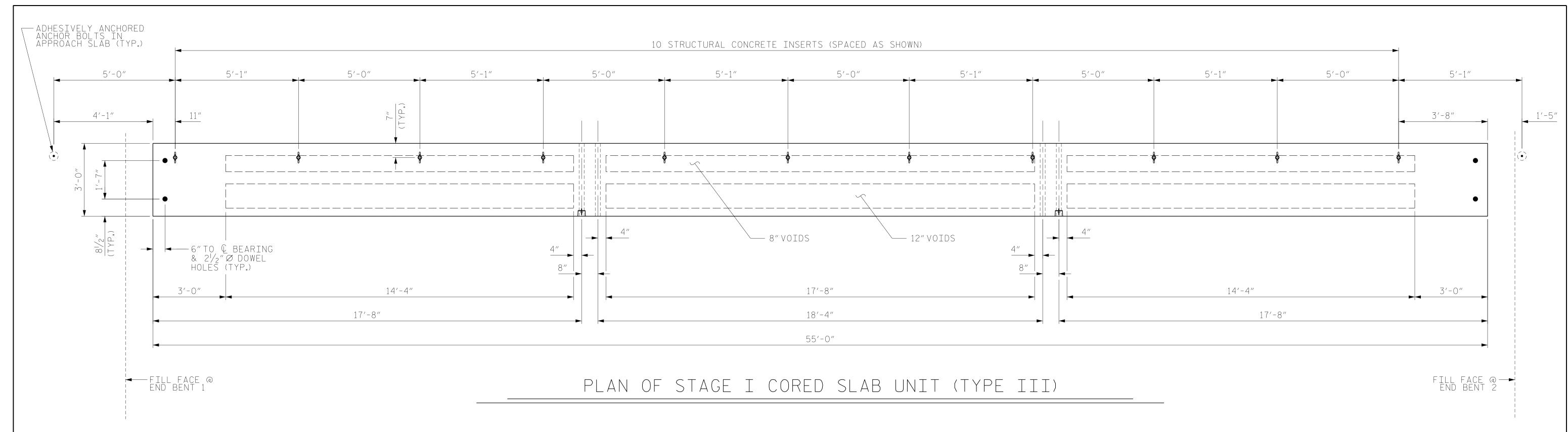
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

CORED SLAB UNIT DETAILS

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919-926-4100 FAX 919-846-9080	1			3			TOTAL SHEETS
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# NOTES

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF  $1\frac{5}{8}$ ".

 $1-\sqrt[]{8}^{\prime\prime}$  Ø X  $8|_2^{\prime\prime}$  Bolt with washer bolt shall conform to the requirements of astm a307. Bolt and washer shall be galvanized. At the contractors option, stainless steel bolt and washer may be used as an alternate for the  $\sqrt[]{8}^{\prime\prime}$  Ø X  $8|_2^{\prime\prime}$  Galvanized bolt and washer. They shall conform to or exceed the mechanical requirements of astm a307. The use of this alternate shall be approved by the engineer.

WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI.

STRUCTURAL CONCRETE INSERT ASSEMBLIES SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

THE COST OF THE STRUCTURAL CONCRETE INSERT ASSEMBLY, COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-0" x 1'-9" PRESTRESSED CONCRETE CORED SLABS.

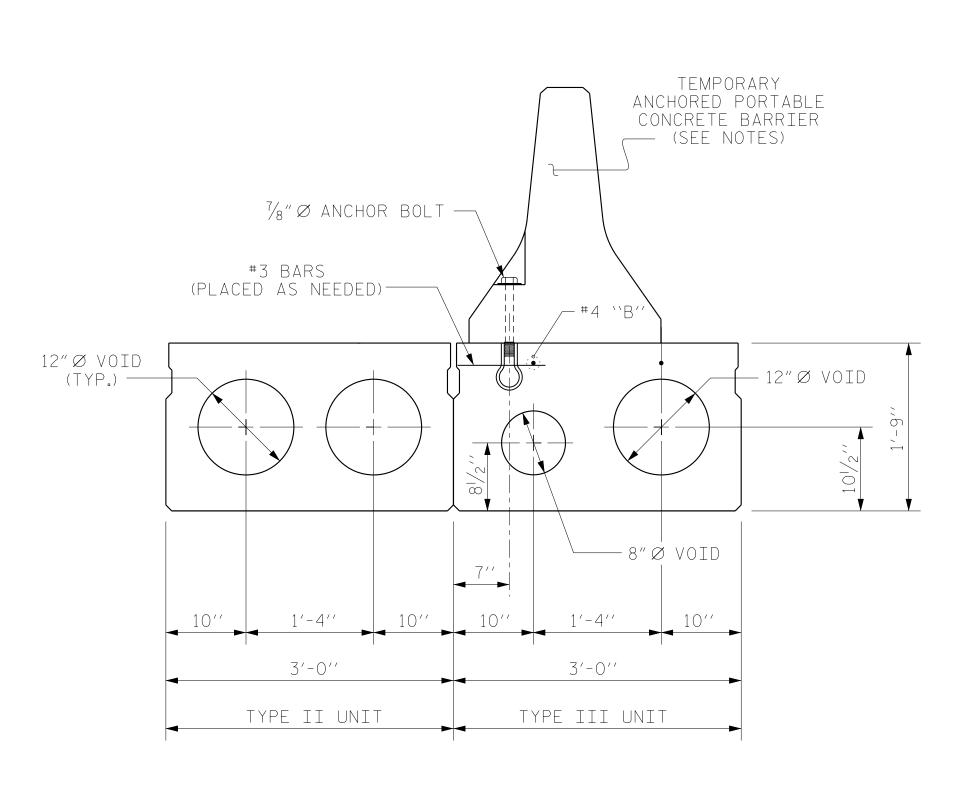
TO FACILITATE PLACEMENT OF STRUCTURAL CONCRETE INSERT ASSEMBLIES, #3 BARS MAY BE TIED TO THE #4 B7 BARS IN THE CORED SLAB UNITS. THE COST OF THE #3 BARS SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-0" x 1'-9" PRESTRESSED CONCRETE CORED SLABS.

STIRRUPS IN THE CORED SLAB UNITS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR STRUCTURAL CONCRETE INSERT ASSEMBLIES.

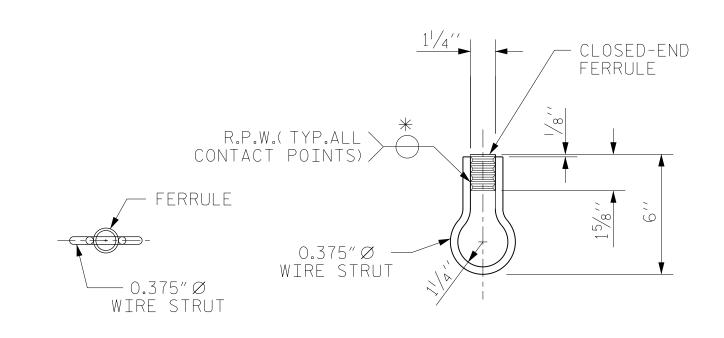
FERRULES TO BE PLUGGED DURING CASTING OF THE CORED SLAB UNITS AS RECOMMENDED BY THE MANUFACTURER.

TEMPORARY ANCHORED PORTABLE CONCRETE BARRIER SHALL BE AS SPECIFIED IN ROADWAY STANDARD NO.1170.01. SEE TRAFFIC CONTROL PLANS.

AFTER REMOVAL OF TEMPORARY ANCHORED PORTABLE CONCRETE BARRIER, THE STRUCTURAL CONCRETE INSERTS SHALL BE FILLED WITH GROUT.



CONCRETE INSERT LOCATION



PLAN

ELEVATION

# STRUCTURAL CONCRETE INSERT

\* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. 17BP.14.R.118 TRANSYLVANIA COUNT STATTON: 13+03.00 -L-

SHEET 5 OF 6



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

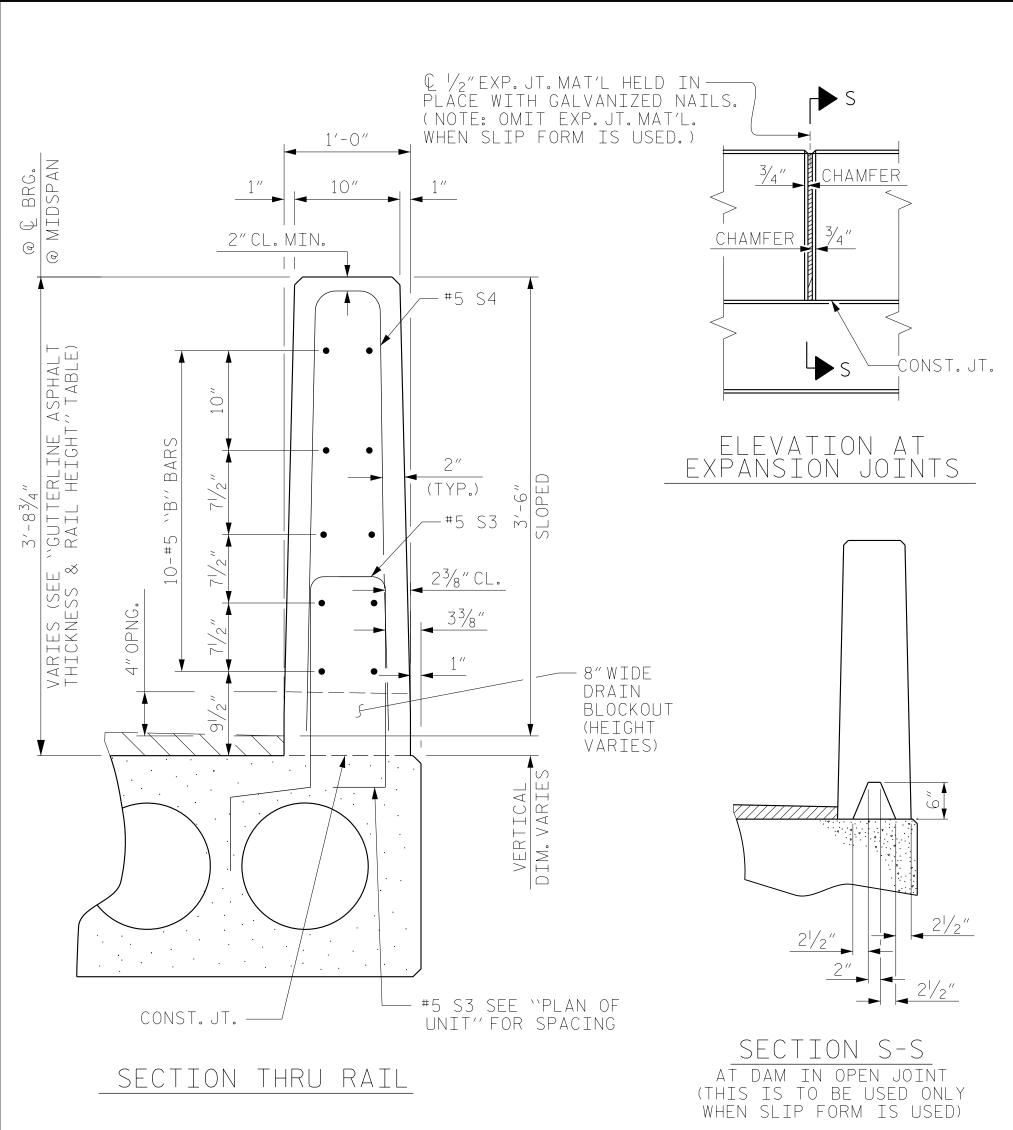
3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
DETAILS

DRAWN BY: TRP DATE: 04/2014
CHECKED BY: JMR DATE: 05/2014
DESIGN ENGINEER OF RECORD: JMR DATE: 05/2014

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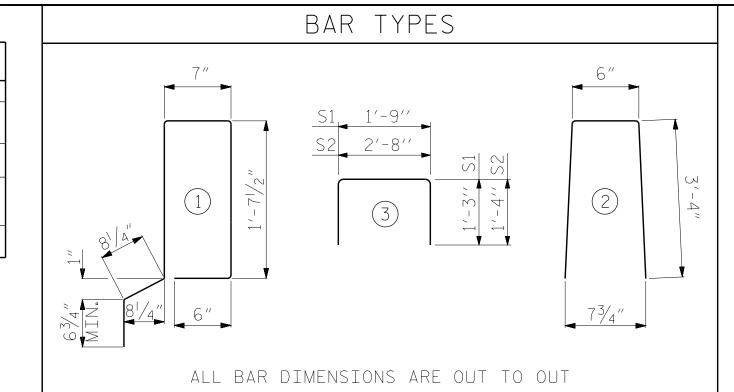
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BY:	DATE:	NO.	BY:	DATE:	S-10
		3			TOTAL SHEETS
		4			21

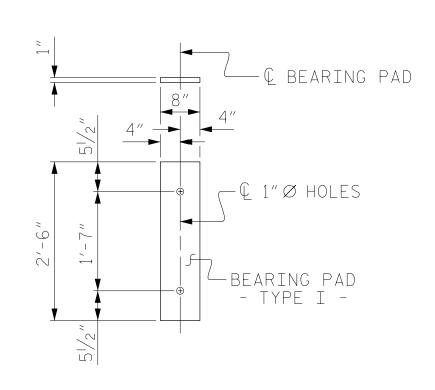


# DEAD LOAD DEFLECTION AND CAMBER 3'-0" x 1'-9" 55' CORED SLAB UNIT CAMBER (SLAB ALONE IN PLACE) DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD \*\*\* INCLUDES FUTURE WEARING SURFACE

GRADE 270 S	TRANDS
	0.6″Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND )	58,600
APPLIED PRESTRESS (LBS. PER STRAND )	43,950

CORED SLABS REQUIRED							
		NUMBER	LENGTH	TOTAL LENGTH			
	TYPE I	1	55'-0"	55'-0"			
STAGE I	TYPE II	4	55'-0"	220'-0"			
JIAGL I	TYPE III	1	55'-0"	55'-0"			
	STAGE I TOTAL	6		330′-0″			
	TYPE IV	3	55′-0″	165′-0″			
STAGE II	TYPE V	1	55′-0″	55'-0"			
	STAGE II TOTAL	4		220'-0"			
TOTAL		10		550'-0"			





FIXED END

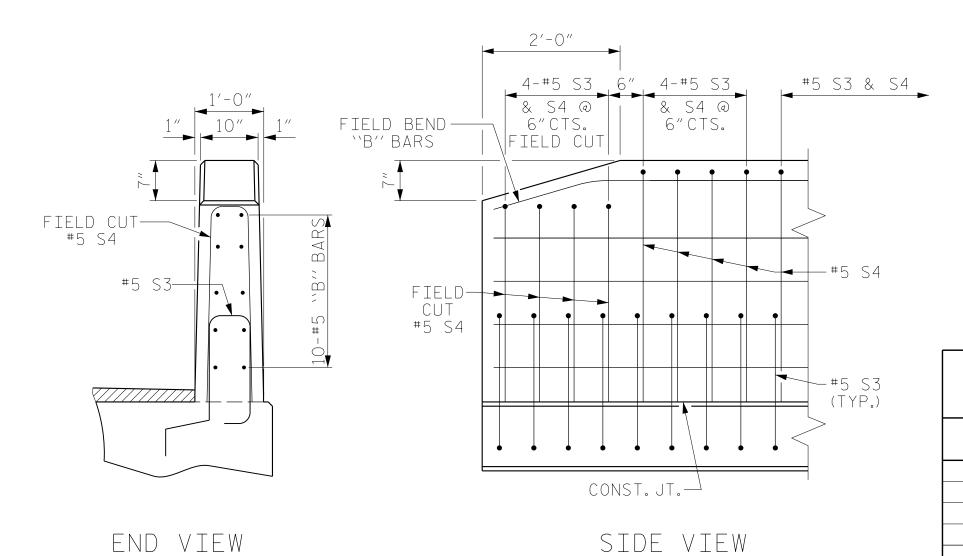
(TYPE I - 20 REQ'D)

# ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

_			
	GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	27'-10" CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
		@ MID-SPAN	@ MID-SPAN
	55'UNITS	15/8″	3'-75/8"

# VERTICAL CONCRETE BARRIER RAIL SECTION



END OF RAIL DETAILS

TRP

JMR

REV. 12/II

ASSEMBLED BY:

DRAWN BY: DGE 5/09

CHECKED BY : BCH 6/09

CHECKED BY :

DATE: 05/2014

DATE: 05/2014

MAA/AAC

MAA/TMG

CONCRETE RELE	ASE STRENGTH
UNIT	PSI
55' UNITS	4900

	BILL OF	ТАМ	ERIA	L FOR	VERT	ICAL	CONCRETE	BARF	RIER	RAIL			
STAGE I							STAGE II						
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT		
* B14	20	#5	STR	27'-1"	565	* B14	20	#5	STR	27'-1"	565		
<b>*</b> S4	64	#5	2	7'-2"	478	* S4	64	#5	2	7'-2"	478		
* EPOXY COATED REINFORCING STEEL LBS. 1043					* EPOX REINFO	Y COATED RCING STEEL			LBS.	1043			
CLASS AA CONCRETE CU.YDS. 7.1					CLASS	AA CONCRETE			CU.YDS.	7.1			
TOTAL Barrie	VERTICAL CON ER RAIL	ICRETE		LN. FT.	55.00	TOTAL BARRIE	VERTICAL CON R RAIL	CRETE		LN. FT.	55.00		

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT													
STAGE I									STAC	GE II			
				TYF	PE I	TYP	EII	TYPE	III	TYPI	EIV	TYP	EV
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT								
В7	4	#4	STR	28'-3"	75	28'-3"	75	28'-3"	75	28'-3"	75	28'-3"	75
S1	8	#5	3	4'-3"	35	4'-3"	35	4'-3"	35	4'-3"	35	4'-3"	35
S2	114	#4	3	5'-4"	406	5'-4"	406	5'-4"	406	5'-4"	406	5′-4″	406
* S3	64	#5	1	5'-7"	373							5′-7″	373
REINFO	RCING STEE	ĒL	LBS.		516		516		516		516		516
* EPOXY COATED LBS. 373										373			
6500 F	S.I. CONCR	ETE	CU. YDS.		7.8		7.8		8.5		7.8		7.8
0.6" Ø	L.R. STRAND	S	No.		19		19		19		19		19

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# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE  $2^{1/2}$   $^{\prime\prime}$   $^{\prime\prime}$  Dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4"X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACHED FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.
STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

PROJECT NO. 17BP.14.R.118
TRANSYLVANIA COUNTY
STATION: 13+03.00 -L-

SHEET 6 OF 6



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STANDARD

3'-0'' X 1'-9''

PRESTRESSED CONCRETE

CORED SLAB UNIT

90° SKEW

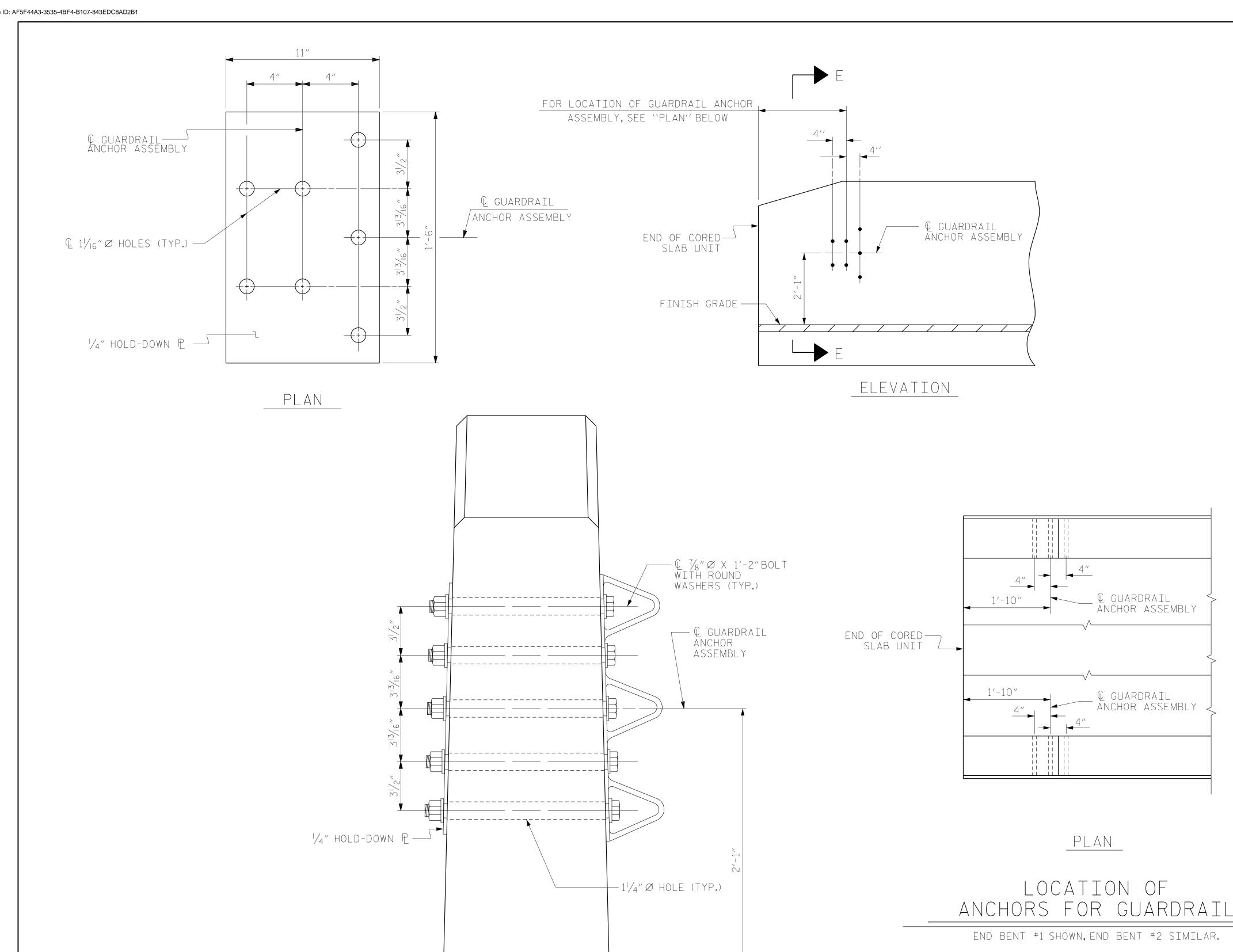
REVISIONS

SHEET NO.

SOLUTION BY: DATE: S-11

TOTAL SHEETS

21



SECTION E-E

\_ DATE : <u>04/2014</u>

DATE : 05/2014

REV. 12/5/II

MAA/GM

MAA/GM

ASSEMBLED BY : \_

CHECKED BY : \_\_\_

DRAWN BY: MAA 5/10

CHECKED BY: GM 5/10

JMR

GUARDRAIL ANCHOR ASSEMBLY DETAILS

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4 HOLD DOWN PLATE AND 7 - 1/8 BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE \( \frac{7}{8}'' \one \text{ GALVANIZED BOLTS,} \) NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



# SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. <u>178P.14</u>.R.118 TRANSYLVANIA COUNTY STATION: 13+03.00 -L-



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STANDARD GUARDRAIL ANCHORAGE

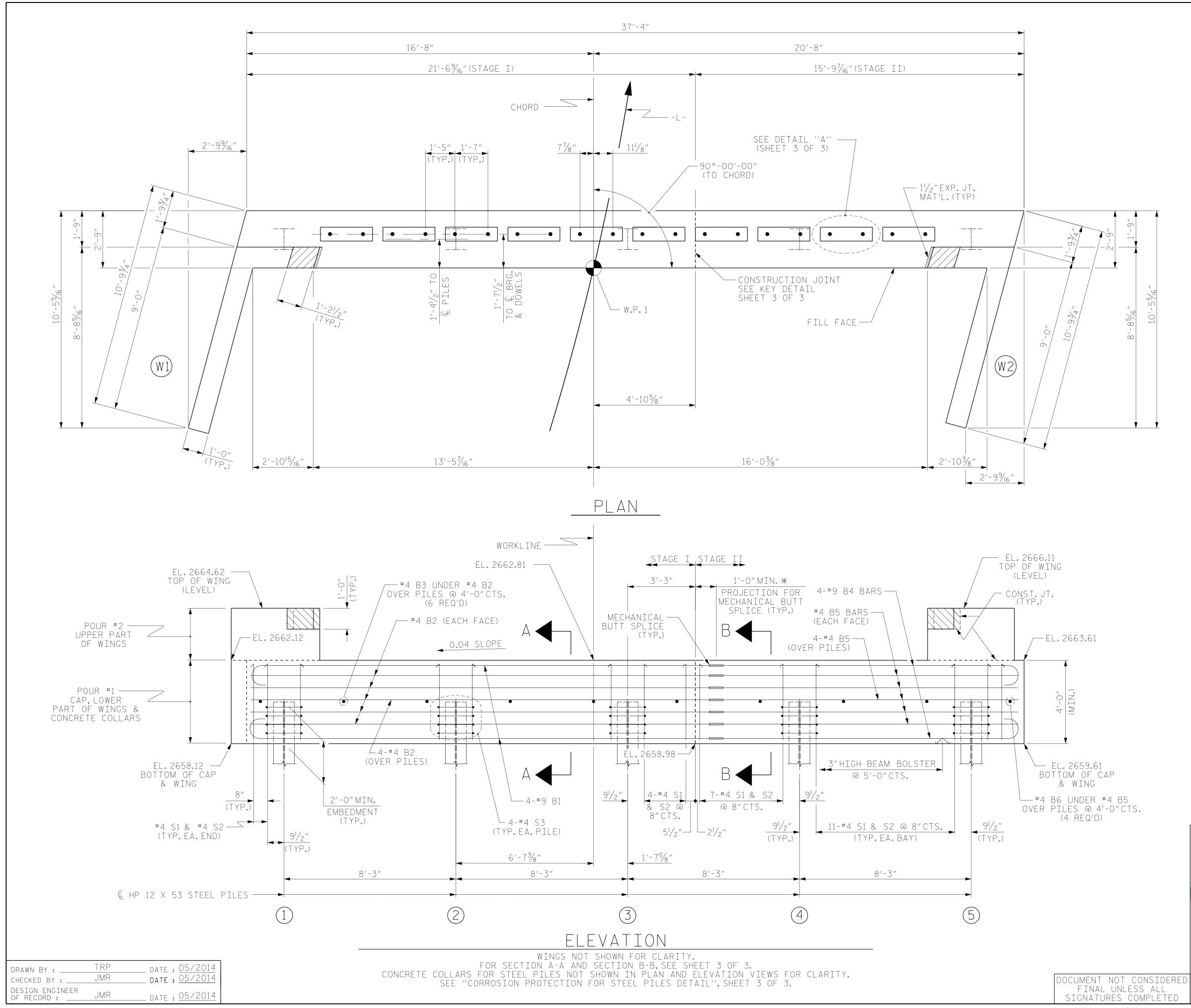
BARRIER RAIL

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TOTAL SHEETS



# NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

FOR WING DETAILS, SEE SHEET 2 OF 3.

\* USE MECHANICAL BUTT SPLICE FOR ALL "B" BARS EXTENDING FROM STAGE 1 CONSTRUCTION JOINT.

#4 BARS MAY BE SPLICED IF A SPLICE OF 2'-5"
CAN BE ACHEIVED. BAR LENGTHS SHOWN IN BILL OF MATERIAL CORRESPOND TO THE USE OF MECHANICAL SPLICES.

FOR MECHANICAL BUTT SPLICES, SEE SECTION 1070-9 OF THE STANDARD SPECIFICATIONS.

TOP OF PILE ELEVATIONS					
1	2660.23				
2	2660.56				
3	2660.89				
4	2661.22				
5	2661.55				

PROJECT NO. 17BP.14.R.118

TRANSYLVANIA COUNTY

STATION: 13+03.00 -L-

SHEET 1 OF 3



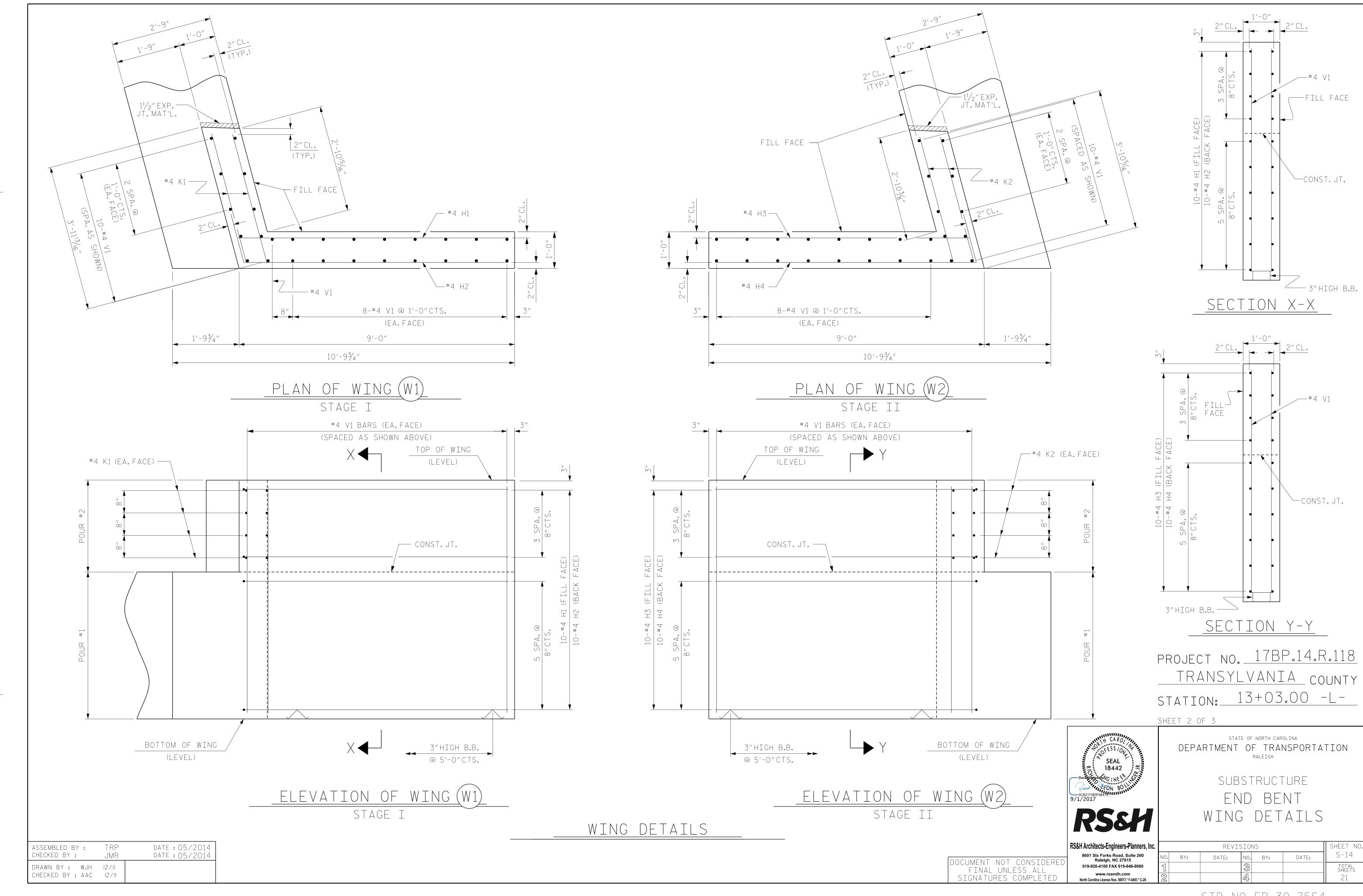
STATE OF NORTH CAROLINA

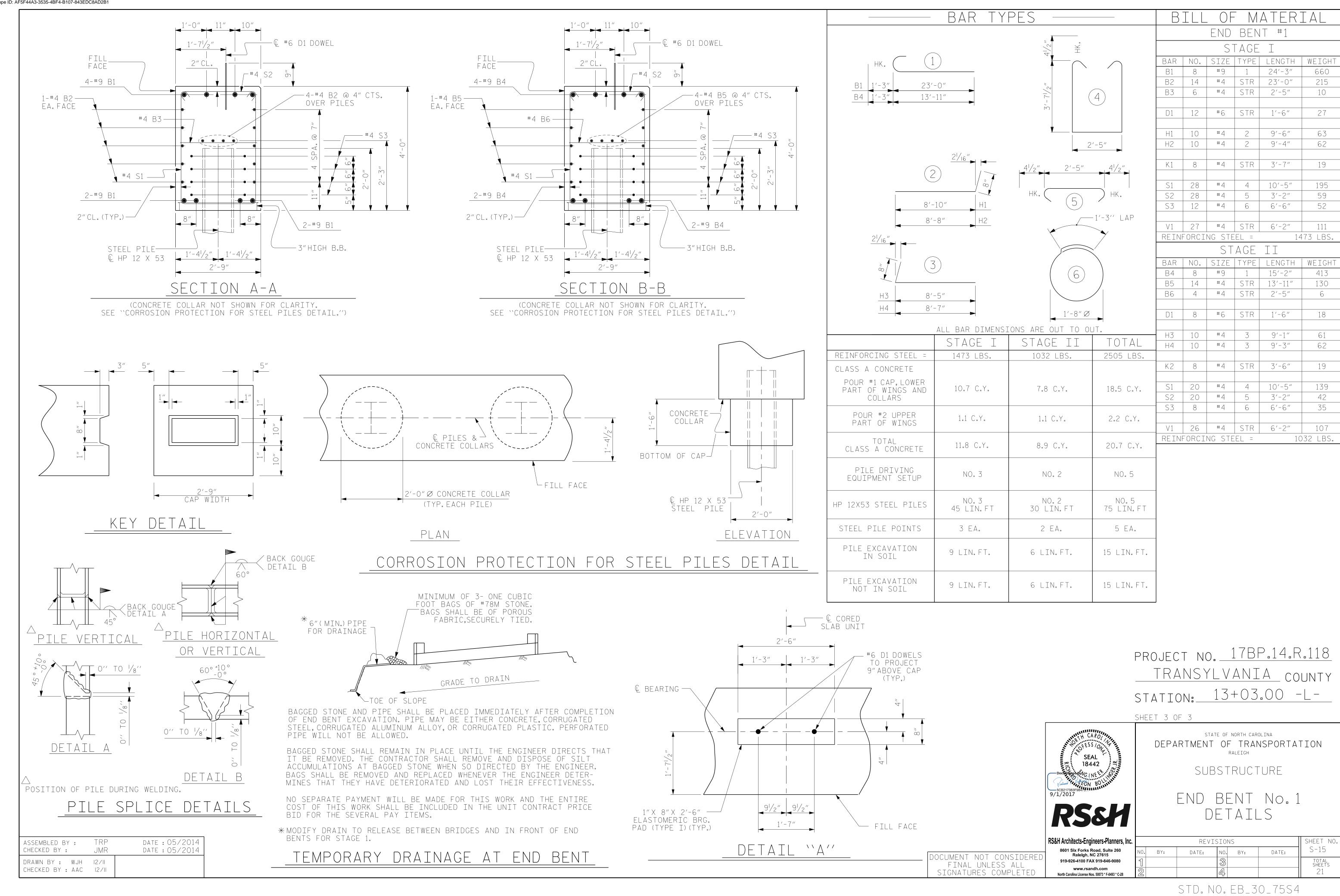
DEPARTMENT OF TRANSPORTATION

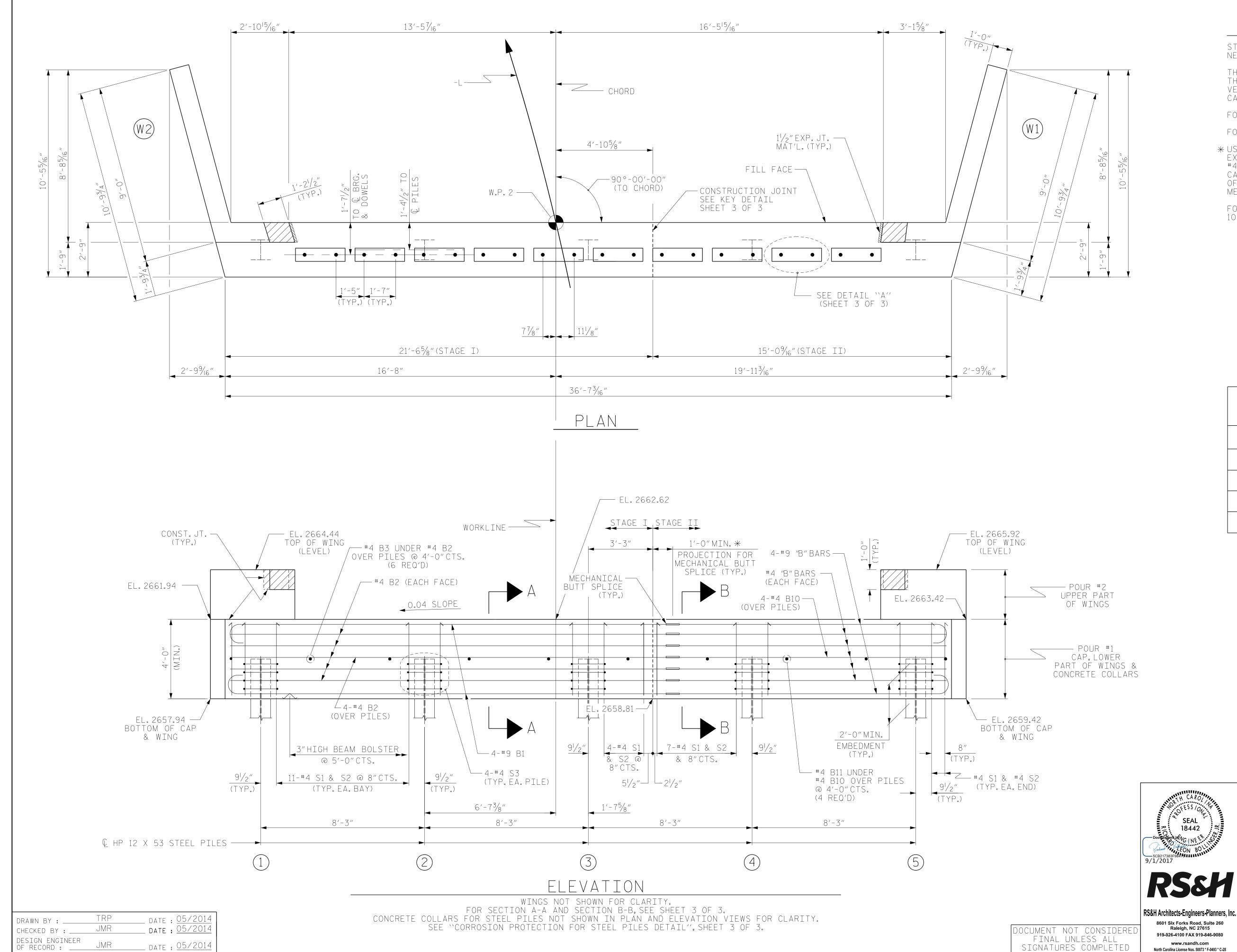
RALEIGH

SUBSTRUCTURE

END BENT No. 1







# NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

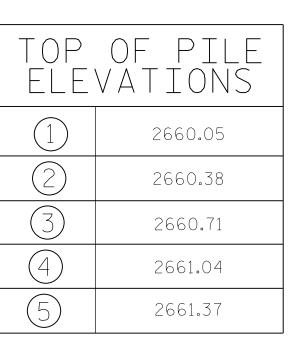
THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

FOR WING DETAILS, SEE SHEET 2 OF 3.

\* USE MECHANICAL BUTT SPLICE FOR ALL "B" BARS EXTENDING FROM STAGE 1 CONSTRUCTION JOINT. #4 BARS MAY BE SPLICED IF A SPLICE OF 2'-5" CAN BE ACHEIVED. BAR LENGTHS SHOWN IN BILL OF MATERIAL CORRESPOND TO THE USE OF MECHANICAL SPLICES.

FOR MECHANICAL BUTT SPLICES, SEE SECTION 1070-9 OF THE STANDARD SPECIFICATIONS.



PROJECT NO. <u>178P.14.R.118</u> TRANSYLVANIA COUNTY STATION: 13+03.00 -L-

SHEET 1 OF 3

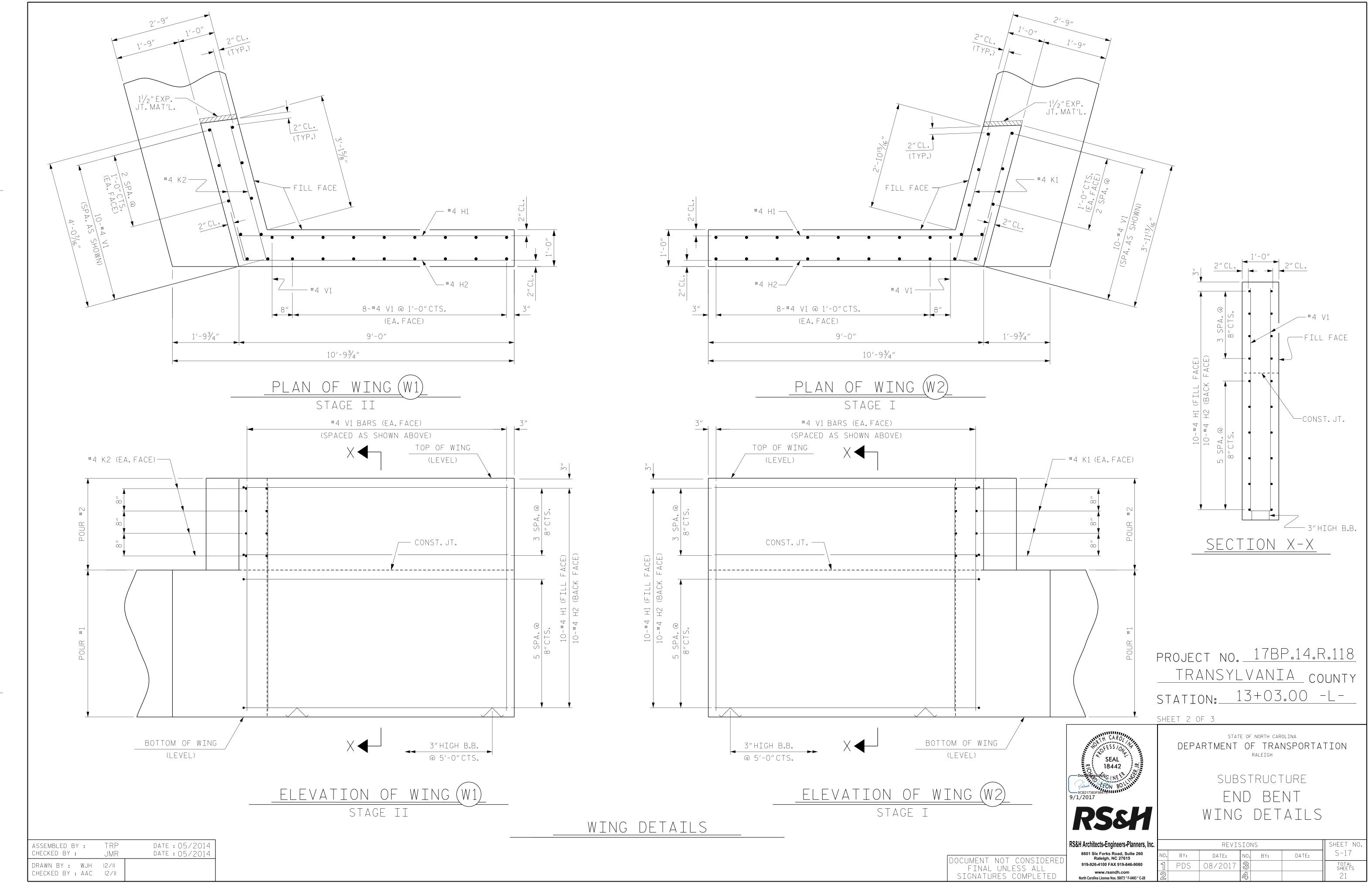


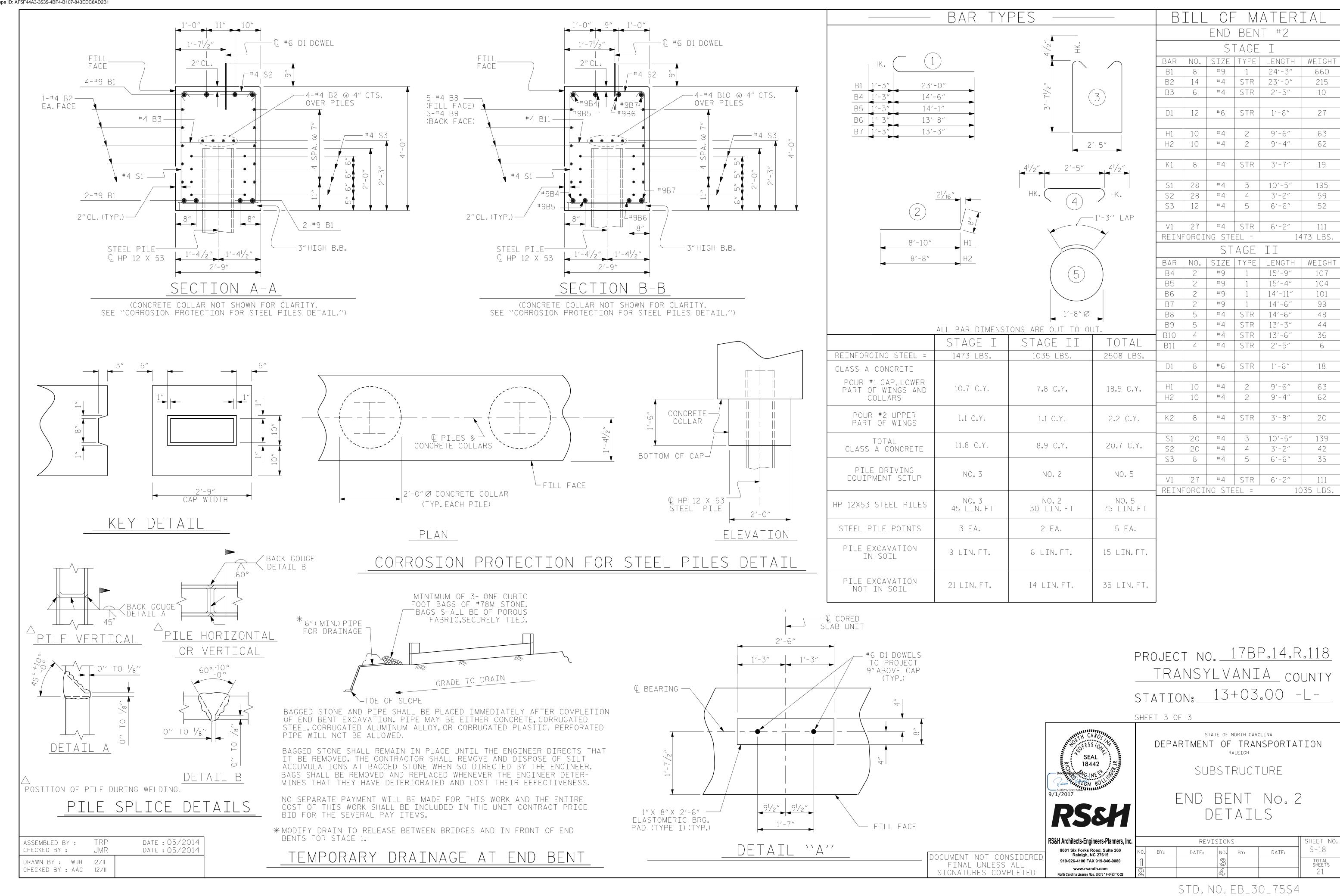
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

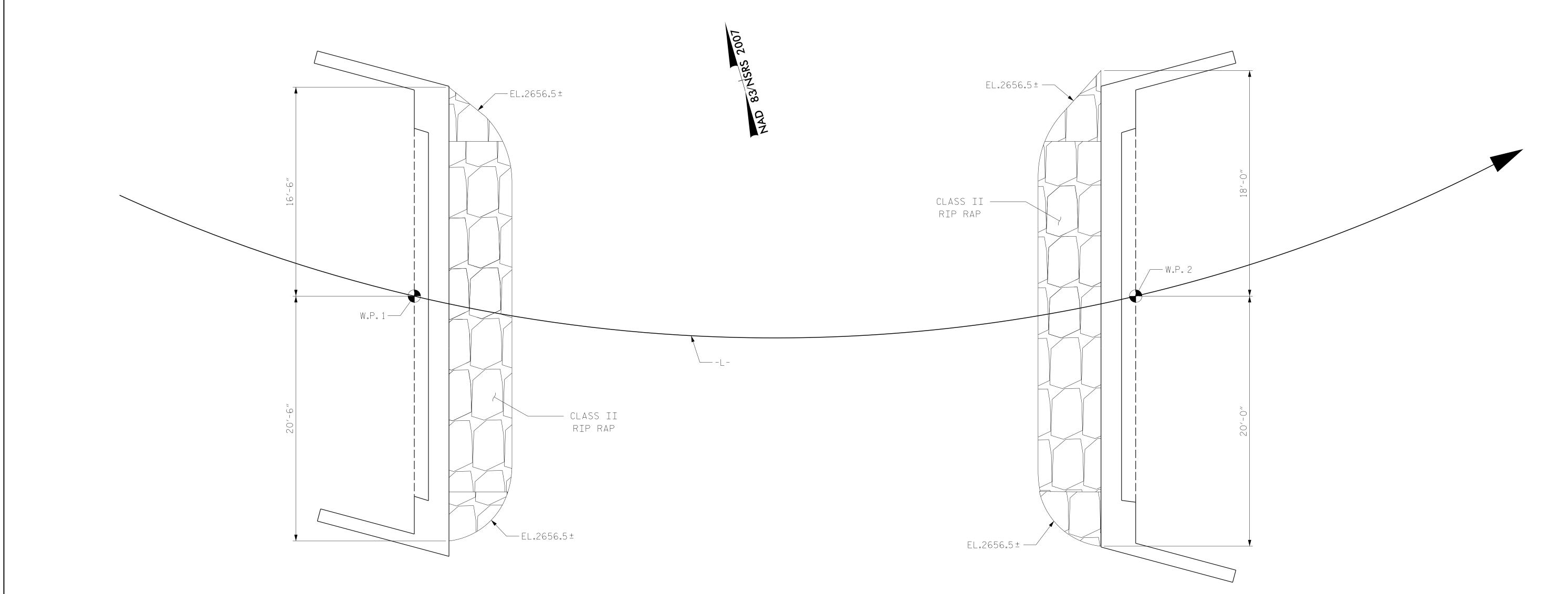
SUBSTRUCTURE

END BENT No. 2

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ESTIMATED QUANTITIES							
BRIDGE @ STA.13+03.00 -L-	RIP RAP CLASS II (3'-6" THICK)	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT 1	38	42.2					
END BENT 2	39	43.3					

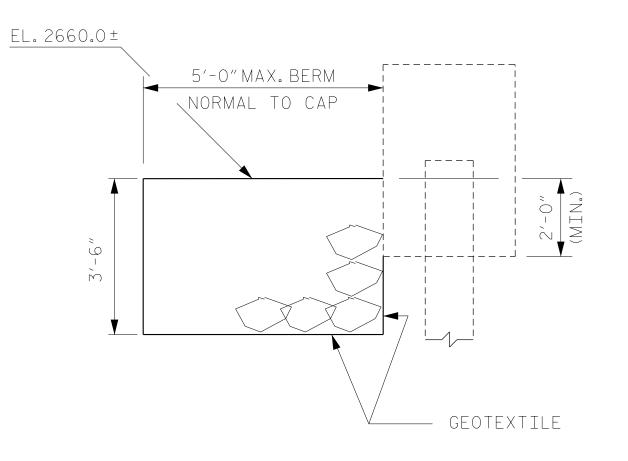
SCEOTEXTILE

5'-0"MAX. BERM
NORMAL TO CAP

"9->EL. 2660.0 ±

Q SECTION @ END BENT NO.1

PLAN



Q SECTION @ END BENT NO.2

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TRANSYLVANIA COUNTY

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SHEET 1 OF 1



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

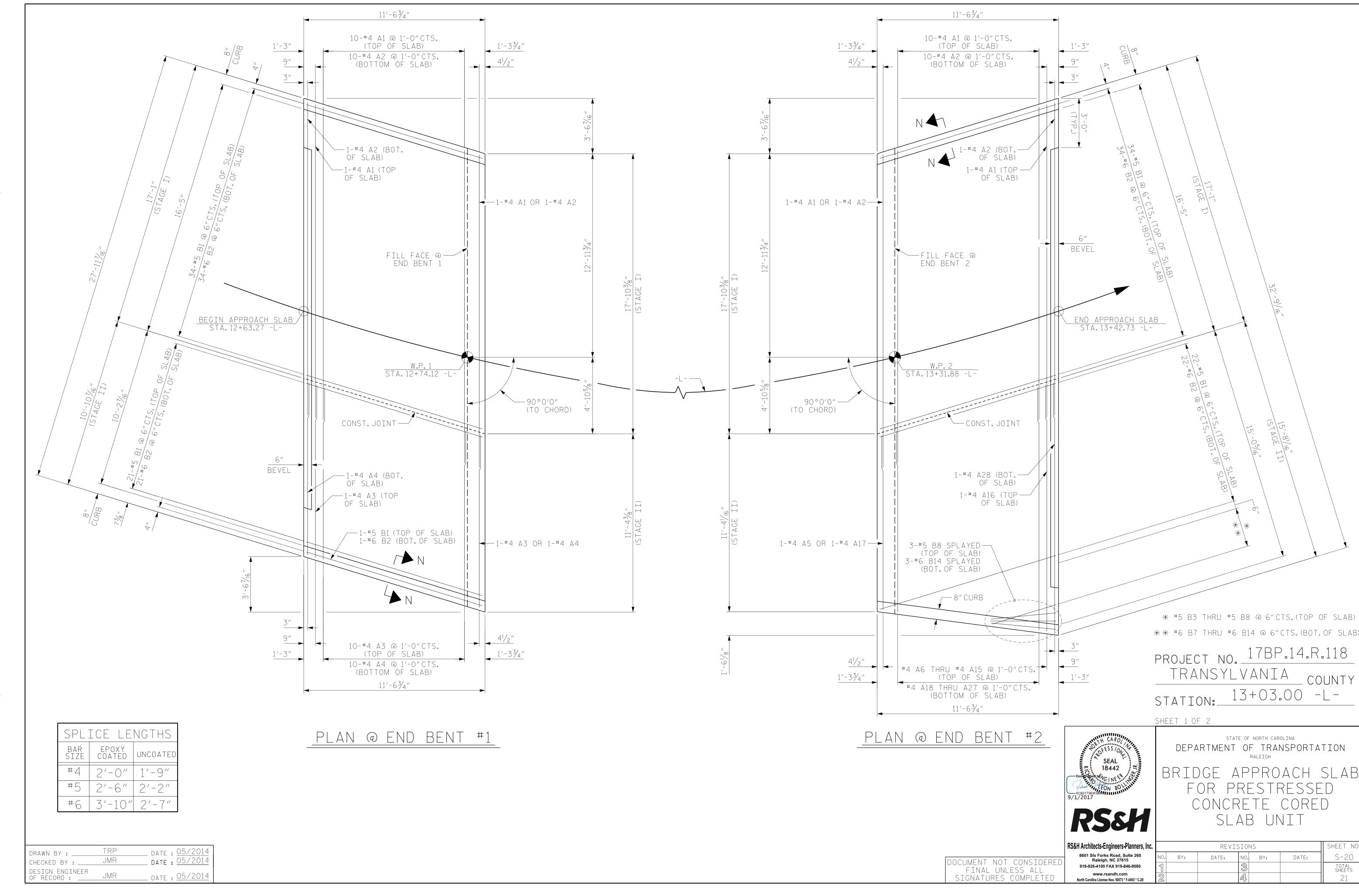
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1 3 TOTAL SHEETS
21

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TRP

JMR

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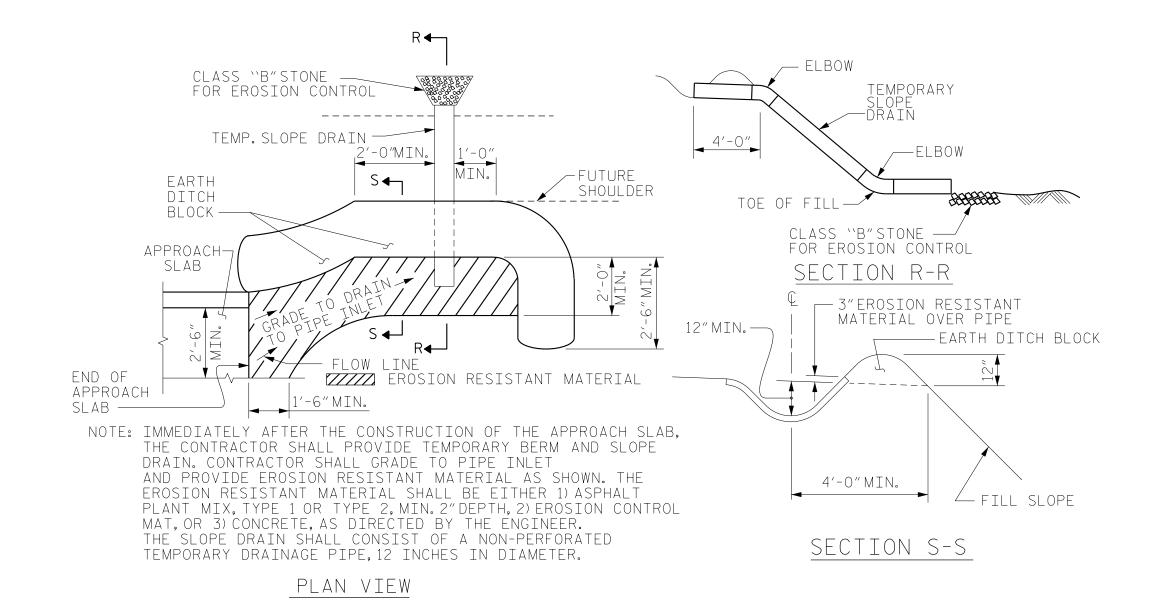
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DATE : 05/2014

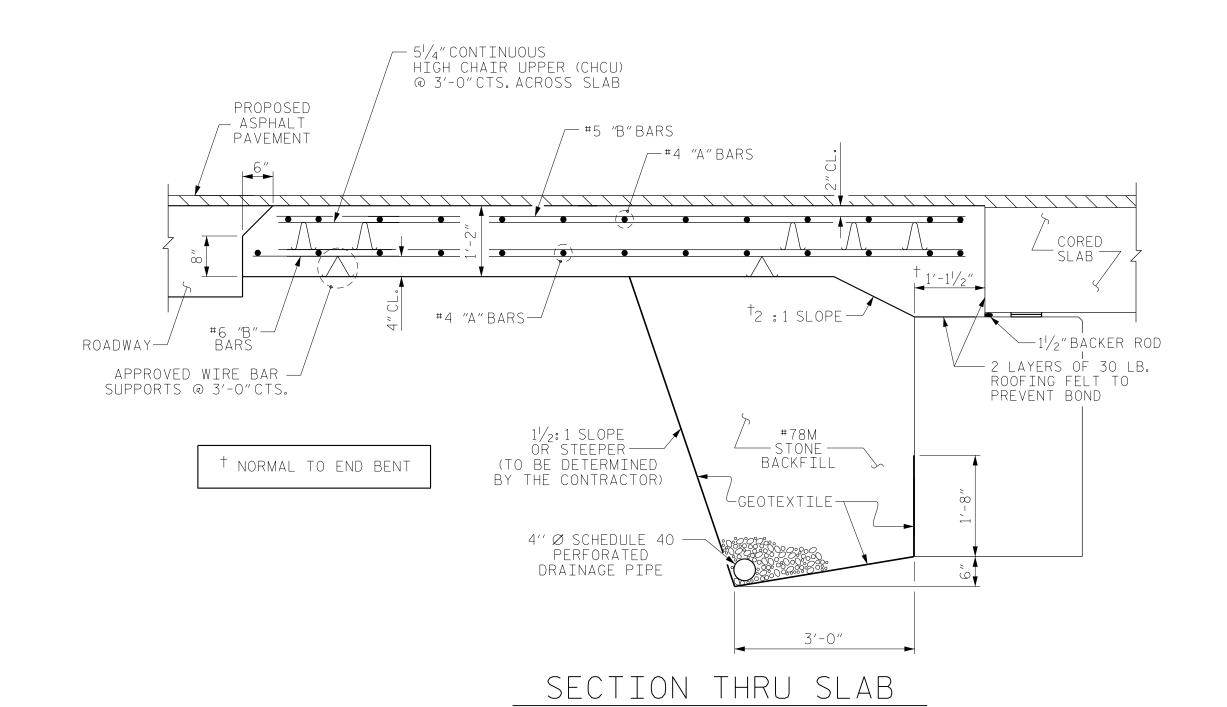
\_ DATE : <u>05/201</u>4

\_ DATE : <u>05/201</u>4

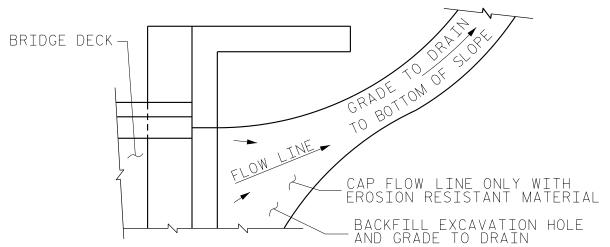


(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

TEMPORARY BERM AND SLOPE DRAIN DETAILS



BILL OF MATERIAL STAGE STAGE II APPROACH SLAB AT EB #1 APPROACH SLAB AT EB #2 APPROACH SLAB AT EB #1 APPROACH SLAB AT EB #2 BAR NO. SIZE TYPE LENGTH WEIGH BAR NO. SIZE TYPE LENGTH WEIGH BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT BAR NO. SIZE TYPE LENGTH WEIGH \* A3 | 12 | #4 | STR | 11'-2" \* A1 | 12 | #4 | STR | 19'-9" 158 \* A1 | 12 | #4 | STR | 19'-9" 158 90 \* A5 | 1 | #4 | STR | 11'-4" A2 | 12 | #4 | STR | 19'-6" A2 | 12 | #4 | STR | 19'-6" A4 12 #4 STR 11'-2" 90 \* A6 | 1 | #4 | STR | 11'-8" 156 156 \* A7 | 1 #4 | STR | 12'-2" \*B1 | 34 | #5 | STR | 11'-2" \* B1 | 34 | #5 | STR | 11'-2" \*B1 | 22 | #5 | STR | 11'-2" **\*** ∆8 | #4 | STR | 12'-7" 396 B2 | 34 | #6 | STR | 11'-8" 596 B2 34 #6 STR 11'-8" 596 B2 | 22 | #6 | STR | 11'-8" 386 \* A9 | #4 | STR | 13'-0" \* A10 1 #4 | STR | 13'-5" #4 | STR | 13′-11″ \* A11 REINFORCING STEEL LBS. REINFORCING STEEL LBS. REINFORCING STEEL LBS. 476 \* A12 #4 | STR | 14'-4" 10 \* EPOXY COATED ₭ EPOXY COATED \* EPOXY COATED REINFORCING STEEL REINFORCING STEEL LBS. REINFORCING STEEL LBS. LBS. #4 | STR | 14′-9″ ¥ A13 10 **★** A14 1 #4 | STR | 15'-2" 10 CLASS AA CONCRETE C.Y. CLASS AA CONCRETE C.Y. 9.3 CLASS AA CONCRETE C.Y. #4 | STR | 15′-8″ 10



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. TEMPORARY DRAINAGE DETAIL

APPROACH SLAB SECTION N-N END OF CURB WITHOUT SHOULDER BERM GUTTER CURB DETAILS

# NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS. GEOTEXTILE SHALL BE TYPE 1IN ACCORDANCE WITH THE STANDARD

SPECIFICATIONS SECTION 1056. #78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN

ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB. FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

PROJECT NO. <u>178P.14.R.118</u> TRANSYLVANIA COUNTY STATION: 13+03.00 -L-

SHEET 2 OF 2

\* A16 1

A17 1

A20 1

A23

A24

A28

#4 | STR | 15′-10″

#4 | STR | 11'-4"

#4 | STR | 11'-8"

#4 | STR | 12'-7"

#4 | STR | 13'-0"

#4 | STR | 13'-5"

#4 | STR | 13′-11″

#4 | STR | 14'-4"

#4 | STR | 14'-9"

#4 | STR | 15'-2"

#4 | STR | 15′-8″

#4 | STR | 15'-10"

#5 | STR | 8'-8"

#5 | STR | 6'-2"

#5 | STR | 5'-5"

#6 | STR | 10′-5″

#6 | STR | 9'-2"

#6 | STR | 7'-11"

#6 | STR | 6'-8"

#5 | STR | 7'-5"

\*B1 | 22 | #5 | STR | 11'-2"

B2 | 22 | #6 | STR | 11'-8"

\*B3 | 1 | #5 | STR | 9'-11"

\*B8 | 4 | #5 | STR | 4'-2"

B13 | 1 | #6 | STR | 5'-5"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

\* EPOXY COATED

B14 | 4 | #6 | STR | 4'-2"

10

10

256

386

10

16

14

25

LBS.

LBS.

C.Y.

#4 STR 12'-2"



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE APPROACH SLAB DETAILS FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 120° SKEW

REVISIONS SHEET NO S-21 DATE: BY: DATE: NO. BY: TOTAL SHEETS

OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

# STANDARD NOTES

# DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) ---- SEE PLANS LIVE LOAD IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27.000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER - - - -

# MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH ----

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

# CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

# DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.

SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

# REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

# HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

# SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH